# Transport Model for Scotland 05A 

 Highways Calibration and Validation Report

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## 1 Introduction

### 1.1 Background

1.1.1 In 2001, MVA was commissioned by the Scottish Executive (now Transport Scotland) to undertake the Transport Model for Scotland (TMfS) project. The purpose was to build on existing transport models (eg CSTM3 and CSTM3A) and develop, support and maintain a methodologically enhanced and geographically expanded multi-modal forecasting tool. The development of TMfS was completed in August 2004.
1.1.2 In December 2005, MVA was instructed by Transport Scotland to undertake a rebase of TMfS to a 2005 Base Year. This work involved the update and enhancement of the model to incorporate newly available data and other procedural enhancements. The model has a Base Year of 2005 and since completion has been used for a range of infrastructure and policy assessments by MVA, other consultants, Local Authorities and Transport Scotland.

### 1.2 Development of TMfS:05A

1.2.1 During 2007, MVA was instructed by Transport Scotland to produce an updated version of TMfS:05, known as TMfS:05A. This model forms the latest version of the model for general release. The main aims of TMfS:05A were twofold.
1.2.2 The first was to improve spatial detail and the representation of the supply side of the model in 'external' areas of the highway and public transport models, mainly the Highlands. The purpose of this enhancement was to feed a new accessibility analysis module to allow nationwide accessibility analysis to be undertaken on a consistent basis. It also provides more accurate travel time / cost information in these areas, building on new journey time surveys also undertaken in 2007.
1.2.3 The second aim was to incorporate new demand data from recent roadside interviews in the highway model in areas where the model was previously perceived to be weak, namely the Ayrshire and Dundee areas.
1.2.4 This report describes the development of the TMfS 2005A Highway Assignment Model (HAM). Separate reports detail the other aspects of the TMfS 2005A development:

■ TMfS:05A PTAM Cal Val Draft Report, MVA October 2007; and
■ TMfS:05A Park and Ride Model Development Report, MVA October 2007.
1.2.5 The TMfS:05A HAM was developed by drawing upon a variety of sources for network information and by incorporating roadside interview survey data and associated traffic count data.
1.2.6 Figure 1.1 illustrates the geographical coverage of the TMfS modelled area.
1.2.7 Throughout this report, the original 2005 TMfS model will be referred to TMfS:05 and the new extended TMfS 2005 model as TMfS:05A.
1.2.8 This report assumes that the reader is familiar with the terminology and processes involved in transport model procedures of this nature. For further information, please refer to the TMfS Website, www.tmfs.org.uk.

### 1.3 Structure of this report

1.3.1 Following this introductory Chapter, this Report includes the following Chapters:

■ Chapter 2 describes the work undertaken on the network development. This covers the updating of all network information and provides a description of the sources used;

- Chapter 3 describes the development of the base year assignment matrices and matrix estimation process used to create the TMfS:05A highway assignment matrices;

■ Chapter 4 describes the development of the TMfS Final Highway Assignment Model and the incorporation of the 'Cost versus Time' Assignment method;

■ Chapter 5 discusses the model calibration data through the presentation of screenline analysis on key strategic routes within the network;

- Chapter 6 discusses the model validation through the presentation of screenline and journey time analysis throughout the modelled network; and
- Chapter 7 provides conclusions and recommendations.


Figure 1.1 TMfS:05A Modelled Area

## 2 Network Development

### 2.1 Introduction

2.1.1 This chapter considers the network developments that have been incorporated into the TMfS:05A model.
2.1.2 The principal developments of the TMfS:05A Highway network are as follows:

■ refinement of modelled road network following the Audit of TMfS:05 and application of model to improve network representation;

- review and refinement of road links in the Highlands and Argyll and Bute areas based on journey time data collected as part of the Strategic Transport Projects Review commission; and
- addition of ferry links and connecting roads infrastructure to represent the Scottish Islands including the Clyde estuary, Argyll and Bute, the western isles, Orkney and Shetland.
2.1.3 The remainder of this chapter is split into the following sections:
- link types;

■ link capacities;

- speed/flow curve definition;
- link distance checks;
- modelled junction data; and
- representation of ferry fares.


### 2.2 Link types

2.2.1 The link types used in the TMfS:05A are in line with those used in the Scottish Transport Statistics Note 24 (see Table 2.1), these remain consistent with TMfS.05. This Link Type numbering system has allowed analysis of model output to be easily compared with published statistics.

Table 2.1 Scottish Transport Statistics Link Type Definitions

| STS Link Type Number | Description |
| :--- | :--- |
| 1 | Trunk - Motorway |
| 2 | Trunk - Motorway slips |
| 3 | Trunk - A Roads Non-Built up |
| 4 | Trunk - A Roads Built up |
| 5 | Non Trunk - A Roads Non-Built up |
| 6 | Non Trunk - A Roads Built up |
| 7 | Minor Roads Non Built up |
| 8 | Minor Roads Built up |

2.2.2 In addition to those link types detailed in Table 2.1, three additional link types have been used in the network:

■ 9 - Banned HGV;
■ 10 - Bus Only; and
■ 14 - Ferry Links (new link type within TMfS:05A).

### 2.3 Link capacities

2.3.1 The link capacities in TMfS:05A remain consistent with those used in TMfS.05.
2.3.2 Table 2.2 highlights the capacities (measured as PCUs per lane and not per carriageway) applied throughout the network. As part of the calibration process, these have also been manually amended in many areas. This process was undertaken to supplement the automated procedure and ensure that capacities provided a more appropriate reflection of conditions.

Table 2.2 Uniform Capacities by Link Type

|  | Link <br> Type <br> 1 | Link <br> Type <br> $\mathbf{1}$ | Link <br> Type <br> $\mathbf{3}$ | Link <br> Type <br> $\mathbf{4}$ | Link <br> Type <br> $\mathbf{5}$ | Link <br> Type <br> $\mathbf{6}$ | Link <br> Type <br> $\mathbf{7}$ | Link <br> Type <br> $\mathbf{8}$ |
| ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Capacity per lane | 2400 | 1800 | 1800 | 1800 | 1600 | 1600 | 1000 | 800 |

### 2.4 Speed/flow curve definition

2.4.1 Table 2.3 presents a descriptive list of TMfS speed/flow curves. These descriptions should not be taken literally but as an indication of the particular speed/flow curve specification. No
changes have been made to the existing speed flow definitions for the updated TMfS:05A from those used in TMfS. 05.
2.4.2 Additional speed/flow curves have, however, been added within TMfS:05A to represent speeds on the journey time routes surveyed as part of the Strategic Transport Projects Review commission. The new capacity indices were based on the current TMfS capacity index 16 (rural routes) with the same profile applied to derive the speed at $80 \%$ capacity, and $100 \%$ capacity. The new curves maintain freeflow speed up to $50 \%$ capacity whereas the speed for capacity index 16 starts to reduce above $0 \%$ capacity. Starting from 26 mph and increasing in steps of 3 mph until 68 mph resulted in new capacity indices 26 to 40 inclusive.

Table 2.3 Speed/Flow Curve and Capacity Index Equivalence List

| TMfS CI | Description |
| :---: | :---: |
|  | City/ Urban Capacity Indices |
| 1 | 40 mph urban road (Tail) |
| 2 | 40 mph urban road (No Tail) |
| 3 | 30 mph urban road (Tail) |
| 4 | 30 mph urban road (No Tail) |
| 5 | 30 mph city centre road (Tail) |
| 6 | 70 mph urban motorway |
| 7 | <70mph urban motorway |
| 8 | 30 mph urban road junction approach |
| 9 | 30 mph city centre road junction approach |
| 10 | Urban expressway |
|  | Suburban Capacity Indices |
| 11 | 30 mph suburban road (Tail) |
| 12 | 30 mph suburban road (No Tail) |
| 13 | Major suburban road |
| 14 | 30 mph suburban road junction approach |
| 15 | >30mph junction approach |
|  | Motorway, Dual, Rural Capacity Indices |
| 16 | Rural routes |
| 17 | Wide single (10m) designed to TD9 |
| 18 | Ramp at grade separated junction |
| 19 | Rural motorway two lanes |
| 20 | Ramp junction approach |
| 21 | Rural motorway three or more lanes |


| TMfS CI | Description |
| :---: | :---: |
| 22 | Rural all purpose three or more lanes |
| 23 | Rural all purpose two lanes |
|  | Other Capacity Indices |
| 24 | Traffic calming |
| 25 | 50mph expressway |
|  | STPR Journey Time Survey Routes |
| 26 | 26 mph freeflow speed |
| 27 | 29 mph freeflow speed |
| 28 | 32 mph freeflow speed |
| 29 | 35 mph freeflow speed |
| 30 | 38 mph freeflow speed |
| 31 | 41 mph freeflow speed |
| 32 | 44 mph freeflow speed |
| 33 | 47 mph freeflow speed |
| 34 | 50 mph freeflow speed |
| 35 | 53 mph freeflow speed |
| 36 | 56 mph freeflow speed |
| 37 | 59 mph freeflow speed |
| 38 | 62 mph freeflow speed |
| 39 | 65 mph freeflow speed |
| 40 | 68 mph freeflow speed |

2.4.3 There are three types of curves are used in the model (see Figure 2.1), which are the same as TMfS.05:

1. conventional - representing link and junction capacity constraints;
2. approach to a node that is not a junction or is not modelled as a junction; and
3. approach to a modelled junction.
2.4.4 Curve One (conventional) has an initial speed up to volume/capacity (V/C) limit and then drops linearly to the speed at capacity. Beyond capacity, it uses the so-called 'DOT 1A Tail' curve. Curve Two uses the same formula to capacity. Beyond capacity, speed is fixed at the capacity speed since on such links, only the link capacity/speed relationship operates, ie the downstream junction capacity is governed by a link with a Type One curve. Curve Three (modelled junction approach) is a fixed speed equal to the free-flow speed. On links approaching modelled junctions, all delay is calculated by the junction modelling delay procedures. The exceptions are that the major arms at a priority junction or the circulating
carriageway on large roundabouts are modelled as a series of priority junctions, which are based on time dependent queuing theory as used in ARCADY/PICADY/OSCADY.
2.4.5 This procedure ensures that intervening 'dummy nodes' (eg for presentation only) do not affect the overall link journey times.
2.4.6 The speed/flow curves used in the TMfS are shown in Table 2.4.

Table 2.4 TMfS speed/flow curves

| Speed / flow curve | Capacity Index |
| :--- | :--- |
| Type 1 | $1,3,5-7,10-11,13,16-19,21-40$ |
| Type 2 | $2,4,12$ |
| Type 3 | $8-9,14-15,20$ |



Figure 2.1
Speed Flow Curve Types

### 2.5 Link distance checks

2.5.1 The link distances for TMfS:05A are analysed in this section. Table 2.5 provides the results of the comparison between the Scottish Transport Statistics Note 24 (STS) and the TMfS:05A base network for Motorway and Trunk A Roads only. The analysis shows there to be a comparable representation of the modelled distance for these strategic link types.

Table 2.5 TMfS:05A Motorway and Trunk A link distance analysis (kms)

| Road Type | STS (S) | TMfS (T) |
| :--- | ---: | ---: |
| Motorway | 383 | 379 |
| Trunk A | 2893 | 2910 |
| Grand Total | $\mathbf{3 2 7 6}$ | $\mathbf{3 2 8 9}$ |

2.5.2 Figures 2.2 and 2.3 show the detail of the TMfS:05A highway network.

### 2.6 Modelled junction data

2.6.1 The modelled junctions within the TMfS:05A Base Highway model are the same as TMfS.05, with the exception of a number of junctions that were updated as part of the network refinement following the TMfS:05 Audit and application of the model.
2.6.2 Appendix A contains the extent of the areas within which modelled junctions are included in the model.

### 2.7 Representation of Ferry Fares

2.7.1 Ferry fares for vehicles have been incorporated within the model through the use of toll files. Vehicle fares were obtained from the ferry operators and the following assumptions were made in order to derive a representation of ferry fares within the model:

- for Caledonian MacBrayne services where multiple ticket types for cars were available an assumed split was derived based on trip purposes extracted from the Origin and Destination of Passengers and Freight on Strategic Sea Crossings Report prepared for HITRANS, Shetland Transport Partnership and Strathclyde Partnership For Transport (March 2007);
- the majority of goods vehicle fares were provided by the ferry operators and an assumed vehicle length of 9 metres was used to represent and average fare;
- modelled car and goods vehicle fares include passenger fares with an assumed vehicle occupancy of 1.5 persons made up of $80 \%$ adults and $20 \%$ children; and
- Orkney and Shetland services from Aberdeen include cabin fares with an assumed split of cabin types being taken.


Figure 2.2 HAM Network Coverage


Figure 2.3 HAM Network (Insert from Figure 2.2)

## 3 Matrix Development

### 3.1 Introduction

3.1.1 Matrix development for TMfS:05A involved enhancing the original TMfS:05 matrices through the following processes:

- conversion to the new TMfS:05A zone structure;
- incorporation of Dundee and Ayrshire road side interview (RSI) data; and

■ matrix estimation.
3.1.2 The remainder of this chapter details the matrix development procedure introduced above: Section 3.2 describes the change in the zone system; Section 3.3 describes the RSI data used and Section 3.4 describes the development of the final matrices prior to matrix estimation while Section 3.5 describes the matrix estimation process used. All figures referred to are presented at the end of the chapter.
3.1.3 To present a comparison of the matrix totals during the stages of development a 14 sector system was developed (see Figure 3.1). This disaggregation of the modelled area facilitates the assessment of changes to the matrix in terms of travel patterns across the TMfS area.

### 3.2 Change in zone system

3.2.1 The TMfS zoning system is based on amalgamations of 2001 Census Output Area Boundaries with the exception of airport zones which are disaggregated further. One of the principal aims in developing TMfS:05A was to improve spatial detail in 'external' areas of model in the Highlands, Argyll and Bute and Islands.
3.2.2 Therefore, zones in these areas have been disaggregated. The main purpose of the zonal disaggregation is to allow a more accurate representation of the costs and times of travel throughout the whole of Scotland, which can be combined with planning data. Zones have been split consistent with census output areas to allow planning data to be built up from census data. The zone disaggregation has been based on a review of significant settlements with all the islands represented individually. Figure 3.2 illustrates the new zones created in TMfS:05A
3.2.3 Figure 3.3 shows the final network wide zoning system. Further details of the model zoning system can be found on the TMfS website (www.tmfs.org.uk).

### 3.3 RSI Data

3.3.1 Three sources of RSI data were used for the development of the TMfS:05A demand matrices as described in Table 3.1 and shown in Figure 3.4. RSI data was also made available by Borders Council, however, this data was not used in the TMfS:05A model development as it was primarily local data and not strategic.

Table 3.1 TMfS:05A New RSI Data

| Dataset | Supplier | Date of Data Collection |
| :--- | ---: | ---: |
| TACTRAN (Dundee and Surrounds) | TACTRAN | March/April 2007 |
| -16 sites |  |  |
| Ayrshire SITM4 -18 sites | SPT/Colin Buchanan |  |
| Kilmarnock -8 sites | East Ayrshire Council 2007 |  |

Note - Kilmarnock data is for morning and evening time periods only.
3.3.2 It should be noted that a further three RSI sites were available within the TACTRAN dataset but these were not used within the TMfS:05A matrix development. The three sites were:

- T14 A85 Riverside Avenue prior to Apollo Way Junction;
- T15 A90 Dundee Kingsway at Gourdy Croft; and

■ T17 A90 South of Forfar at Gallowfauld.
3.3.3 At all these sites, recent RSI data had been incorporated in TMfS. A select link analysis was undertaken at the site locations. This was compared with the new RSI site data, which showed very similar travel patterns. Therefore, it was not considered that new RSI data would benefit the TMfS:05A matrix. Count information for these three sites was, however, used in the matrix estimation and calibration of the model as described later in this report.

## RSI Data Processing

3.3.4 The processing of the RSI data included a number of data checking and cleaning tasks. Initial mapping of the origin and destination points for the records at the TACTRAN and Kilmarnock RSI sites indicated a significant number of illogical movements - typically between $5 \%$ and $30 \%$ for TACTRAN sites and around $10 \%$ for Kilmarnock sites. Therefore, given the volume of illogical records, it was necessary to undertake data cleaning in a database. Sectors were defined and allocated to each origin and destination for each record using GIS. Illogical sector movements were then identified for each site. The data was then processed using a database and illogical movements were discarded prior to calculating expansion factors for each site. The SITM4 Ayrshire RSI data did not contain the same volume of illogical movements (typically less than 5\%) and data cleaning was undertaken manually through visual inspection of the records.
3.3.5 Following the data cleaning, vehicle matrices were prepared for each individual RSI site as follows:

- append TMfS:05A origin zone and destination zone attributes to each RSI record;
- aggregate the RSI records to form interview direction record matrices for each time period and user class - AM includes records between 0700-1000 hours, IP 1000-1600 hours, PM 1600-1900 hours;
- transpose the interview direction record matrices to create reverse direction matrices AM interview matrices transposed to represent the PM reverse, IP interview matrices transposed to represent the IP reverse, PM interview matrices transposed to represent the AM reverse;
- calculate matrix expansion factors for each time period and vehicle type based on the record matrix totals and the corresponding count data;

■ expand each time period / user class / direction matrix to the observed count, using the calculated expansion.
3.3.6 The 2006/2007 counts were scaled back to 2005 estimated counts using factors derived from Scottish Transport Statistics.
3.3.7 Two TACTRAN sites in central Dundee had no interpeak count data and factors derived from neighbouring sites were applied to equivalent AM peak counts at the sites.
3.3.8 The RSI site on the A78 ( T ) south of Pennyburn had missing data in the evening peak and, therefore, the interpeak and evening records where combined before calculating the expansion factors.
3.3.9 At a number of sites the expansion factors for heavy goods vehicles and to a lesser extent light goods vehicles were very high as few RSI records were available. In these instances data patching was undertaken and records from neighbouring sites were copied to obtain a better representation of the travel pattern.

### 3.4 Prior Matrix Development

3.4.1 The prior matrix for TMfS:05A was developed by combining the TMfS:05 matrix with the new RSI data.
3.4.2 TMfS:05 matrices were first converted to the new zoning system as described in section 3.2. This was undertaken using population data for the new zones where the travel pattern was retained from the TMfS:05 demand matrix and the volume of trips was split proportionate to the population totals.
3.4.3 The RSI matrices were combined with TMfS:05 matrices as follows.

■ For the TACTRAN RSI sites on the A93 south of Blairgowrie and on the A94 north of Scone Airport, select link matrices were derived from TMfS:05 and these trips were removed in the TMfS:05 matrix and replaced with the RSI data;

- For the TACTRAN RSI site on the A90 North of Forfar, trips between Forfar and zones south of Aberdeen were selected for the RSI matrix and replaced the equivalent movements in the TMfS:05 matrix;

■ The 13 RSI sites in Dundee formed a fully observed cordon, which replaced the equivalent movements in the TMfS:05 matrix;

- The 18 RSI sites in Ayrshire were combined into five screenlines and fully observed movements were identified for each. The screenlines were then combined to form a complete observed Ayrshire RSI matrix and potential multiple observed movements were factored to obtain the average number of trips across the screenlines. The TMfS:05 matrix was then replaced with the Ayrshire RSI matrix for fully observed movements; and

■ The 8 RSI sites in Kilmarnock formed a fully observed cordon, which replaced the equivalent movements in the TMfS:05 matrix.
3.4.4 The above procedures were carried out in five discrete stages where the matrix output from Stage 1 was the input matrix to Stage 2 and so on. This avoided potential double counting of RSI data.

### 3.5 Matrix Estimation

3.5.1 The calibration of the assignment process was undertaken using the CUBE based Matrix Estimation program MVESTM.
3.5.2 MVESTM uses a wide variety of data sources, each of which has a confidence level assigned to it. Through this approach, it is possible to manipulate MVESTM to make changes in the areas where the expressed level of confidence is lower. This feature was used to estimate the 2005 HAM matrices and used the following data:

- prior matrix (with a confidence of 100 for TMfS:05 movements and 75 for TMfS:05A RSI movements);
- trip end data (with a confidence of 40);
- paths; and
- traffic counts (with an initial confidence of 100 for counts used to develop TMfS:05 and 75 for new TMfS:05A RSI counts).
3.5.3 It should be noted that TMfS.05A has been developed to a 2005 base year, however, the new RSI data and associated count data is from 2006 and 2007, albeit scaled back to 2005 traffic levels. In order to retain the existing travel pattern in areas away from the new RSI sites a higher confidence interval was associated with the old matrix and count data.
3.5.4 MVESTM requires a set traveller paths from the model. The trip points used in the estimation process were representative of the best paths available after a run of the model with a new matrix. MVESTM and the traffic model were run iteratively with successively improving paths and costs being fed into the MVESTM program. 'Burrell paths' were built after each modelled time period achieved convergence following capacity restraint assignment. MVESTM was provided with three sets of paths built separately for each time period after the last iteration of assignment. It was considered that these were most appropriate as they were shown to represent stable network conditions.
3.5.5 The count data used for the estimation process was as per that used to develop TMfS:05 with the addition of the TMfS:05A RSI counts. From these count locations, count screenlines were created for use in MVESTM. Appendix B contains graphical representations of the locations of the screenlines used in calibration.


### 3.6 Matrix Development Comparisons

3.6.1 Tables 3.2 to 3.13 detail the peak hour matrix totals for the TMfS.05, TMfS.05A Prior Incorporating RSIs (Prior meaning before MVESTM) and Final Highway matrix totals. For all analysis, the matrix values are in PCUs $\times 10$.
3.6.2 Inspection of the sector matrices indicates the following points of interest:

- inclusion of the RSI data in the prior matrix increases the volume of trips within Ayrshire in all three time periods with a reduction in trips to external sectors and a net increase overall;
- inclusion of the RSI data in the prior matrix increase the volume of trips to/from Dundee in the morning and inter peak periods with a reduction in the evening peak; and
- overall, the change in the matrix from the prior matrices to the final post-MVESTM matrices in absolute terms is small.

Table 3.2 AM Peak Hour TMfS:05 Matrix (PCUs x 10)

| OD | 1 | 2 | 3 | 4 | 5 | 6 | 7 | 8 | 9 | 10 | 11 | 12 | 13 | 14 | Total |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | 452336 | 71949 | 10736 | 8101 | 4155 | 7384 | 1078 | 1180 | 2751 | 3143 | 210 | 1641 | 1243 | 1891 | 567798 |
| 2 | 86997 | 84014 | 6575 | 13164 | 5273 | 12619 | 1582 | 1105 | 2768 | 1814 | 161 | 1674 | 386 | 3170 | 221303 |
| 3 | 12943 | 9624 | 229717 | 11239 | 1376 | 2707 | 282 | 50 | 188 | 15202 | 13023 | 5307 | 3749 | 591 | 305997 |
| 4 | 14417 | 19347 | 9616 | 196687 | 18471 | 28423 | 1896 | 212 | 244 | 4388 | 556 | 3085 | 849 | 830 | 299020 |
| 5 | 6141 | 6699 | 1067 | 11819 | 682497 | 159648 | 9726 | 1042 | 1292 | 550 | 359 | 390 | 1710 | 1606 | 884546 |
| 6 | 12201 | 17424 | 3152 | 22295 | 251642 | 503457 | 24299 | 3258 | 450 | 1392 | 1048 | 1105 | 4056 | 3809 | 849588 |
| 7 | 1208 | 2052 | 149 | 2109 | 13088 | 30240 | 105759 | 1300 | 446 | 240 | 13 | 223 | 11656 | 1956 | 170439 |
| 8 | 2675 | 1253 | 69 | 657 | 901 | 2926 | 2810 | 82541 | 1823 | 32 | 0 | 15 | 1925 | 2206 | 99835 |
| 9 | 8943 | 4253 | 179 | 219 | 3698 | 485 | 362 | 1292 | 10494 | 40 | 0 | 46 | 28 | 6114 | 36154 |
| 10 | 1812 | 2351 | 10716 | 7393 | 1482 | 1999 | 291 | 164 | 40 | 22854 | 10201 | 4392 | 4622 | 1361 | 69678 |
| 11 | 683 | 283 | 4710 | 398 | 238 | 248 | 1 | 221 | 0 | 8289 | 35552 | 12022 | 111 | 647 | 63405 |
| 12 | 587 | 1059 | 3651 | 711 | 692 | 1026 | 236 | 45 | 51 | 5315 | 21338 | 364563 | 2636 | 2876 | 404786 |
| 13 | 139 | 225 | 594 | 159 | 4834 | 2018 | 4301 | 1 | 293 | 3691 | 613 | 591 | 502 | 295 | 18256 |
| 14 | 1827 | 2099 | 643 | 271 | 2187 | 1212 | 2422 | 756 | 2609 | 140 | 82 | 562 | 164 | 0 | 14975 |
| Total | 602909 | 222631 | 281574 | 275224 | 990535 | 754393 | 155046 | 93166 | 23449 | 67091 | 83157 | 395614 | 33635 | 27354 | 4005779 |

Table 3.3 AM Peak Hour Prior Matrix Before MVESTM (PCUs x 10)

| OD | 1 | 2 | 3 | 4 | 5 | 6 | 7 | 8 | 9 | 10 | 11 | 12 | 13 | 14 | Total |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | 452302 | 71949 | 10736 | 8101 | 4155 | 7384 | 410 | 1180 | 2751 | 3142 | 390 | 1393 | 1136 | 1891 | 566919 |
| 2 | 86997 | 83747 | 6575 | 13164 | 5273 | 12619 | 695 | 1105 | 2768 | 1779 | 335 | 1761 | 406 | 3170 | 220394 |
| 3 | 12943 | 9624 | 229328 | 11239 | 1376 | 2707 | 387 | 50 | 188 | 15790 | 15506 | 5394 | 3691 | 591 | 308814 |
| 4 | 14417 | 19347 | 9616 | 196271 | 18471 | 28423 | 1159 | 212 | 244 | 4397 | 410 | 3031 | 915 | 830 | 297743 |
| 5 | 6141 | 6699 | 1067 | 11819 | 682340 | 159648 | 10912 | 1042 | 1292 | 549 | 299 | 270 | 1672 | 1606 | 885358 |
| 6 | 12201 | 17424 | 3152 | 22295 | 251642 | 502899 | 18899 | 3191 | 450 | 1450 | 707 | 621 | 4015 | 3809 | 842755 |
| 7 | 1038 | 1161 | 345 | 1045 | 14914 | 19308 | 153514 | 1195 | 389 | 231 | 22 | 244 | 4180 | 1097 | 198682 |
| 8 | 2675 | 1253 | 69 | 657 | 901 | 2915 | 1996 | 82541 | 1823 | 25 | 0 | 20 | 1904 | 2206 | 98986 |
| 9 | 8943 | 4253 | 179 | 219 | 3698 | 485 | 69 | 1292 | 10494 | 40 | 22 | 46 | 34 | 6114 | 35888 |
| 10 | 1827 | 2273 | 10618 | 7374 | 1517 | 1987 | 379 | 164 | 40 | 24950 | 10738 | 2740 | 5550 | 1415 | 71572 |
| 11 | 765 | 255 | 4784 | 345 | 214 | 426 | 25 | 221 | 0 | 5732 | 44419 | 12844 | 146 | 684 | 70859 |
| 12 | 593 | 1062 | 3669 | 645 | 658 | 714 | 31 | 45 | 51 | 3810 | 23265 | 365489 | 2634 | 2914 | 405581 |
| 13 | 199 | 284 | 735 | 246 | 4798 | 1997 | 1918 | 24 | 187 | 5863 | 758 | 588 | 98 | 300 | 17997 |
| 14 | 1827 | 2099 | 643 | 271 | 2187 | 1212 | 1514 | 756 | 2609 | 207 | 104 | 657 | 96 | 0 | 14181 |
| Total | 602869 | 221428 | 281516 | 273693 | 992145 | 742725 | 191907 | 93016 | 23286 | 67964 | 96975 | 395098 | 26478 | 26629 | 4035730 |

Table 3.4 AM Peak Hour TMfS:05A Final Matrix (PCUs x 10)

| OD | 1 | 2 | 3 | 4 | 5 | 6 | 7 | 8 | 9 | 10 | 11 | 12 | 13 | 14 | Total |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | 453382 | 66495 | 10483 | 7788 | 3986 | 7117 | 373 | 1172 | 2470 | 3367 | 321 | 947 | 943 | 1613 | 560457 |
| 2 | 86317 | 80187 | 6440 | 13118 | 5180 | 12776 | 619 | 1085 | 2500 | 1909 | 279 | 1389 | 232 | 2781 | 214813 |
| 3 | 11462 | 6852 | 229096 | 10852 | 943 | 2127 | 216 | 31 | 129 | 15610 | 14817 | 5067 | 4160 | 366 | 301727 |
| 4 | 12732 | 16318 | 9366 | 197292 | 17043 | 27641 | 872 | 193 | 199 | 4658 | 330 | 2799 | 709 | 685 | 290837 |
| 5 | 5019 | 5813 | 933 | 12187 | 690674 | 160205 | 9763 | 974 | 1255 | 581 | 278 | 218 | 1494 | 1318 | 890710 |
| 6 | 10793 | 16233 | 2912 | 22476 | 240423 | 502867 | 21827 | 3129 | 435 | 1582 | 672 | 548 | 3743 | 3223 | 830864 |
| 7 | 850 | 1081 | 262 | 972 | 12225 | 21619 | 161519 | 1290 | 613 | 180 | 17 | 159 | 3987 | 1061 | 205834 |
| 8 | 2643 | 1105 | 61 | 622 | 708 | 2645 | 2219 | 82863 | 1933 | 23 | 0 | 16 | 1706 | 2379 | 98921 |
| 9 | 8892 | 3823 | 154 | 218 | 3121 | 507 | 87 | 1319 | 12339 | 36 | 22 | 35 | 20 | 6146 | 36718 |
| 10 | 1602 | 1571 | 11264 | 7225 | 1236 | 1730 | 262 | 131 | 27 | 25058 | 11343 | 3247 | 5102 | 966 | 70764 |
| 11 | 692 | 153 | 4559 | 431 | 193 | 414 | 14 | 151 | 0 | 6858 | 44671 | 13249 | 191 | 519 | 72094 |
| 12 | 435 | 550 | 3310 | 675 | 468 | 510 | 17 | 33 | 29 | 4305 | 22832 | 340132 | 2909 | 1960 | 378166 |
| 13 | 140 | 164 | 792 | 262 | 4144 | 3050 | 3105 | 45 | 129 | 5762 | 1239 | 574 | 74 | 236 | 19718 |
| 14 | 1902 | 2180 | 650 | 278 | 2093 | 1396 | 1361 | 781 | 2789 | 228 | 87 | 552 | 89 | 0 | 14387 |
| Total | 596859 | 202524 | 280283 | 274394 | 982436 | 744604 | 202253 | 93196 | 24848 | 70159 | 96908 | 368931 | 25360 | 23254 | 3986009 |

Table 3.5 Inter Peak Hour TMfS:05 Matrix (PCUs x 10)

| OD | 1 | 2 | 3 | 4 | 5 | 6 | 7 | 8 | 9 | 10 | 11 | 12 | 13 | 14 | Total |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | 352047 | 55662 | 5830 | 6512 | 4442 | 6267 | 614 | 1689 | 3247 | 1156 | 303 | 1036 | 986 | 2278 | 442070 |
| 2 | 42695 | 57806 | 6647 | 9527 | 3539 | 11013 | 1246 | 624 | 2584 | 1818 | 808 | 1671 | 661 | 3529 | 144169 |
| 3 | 6995 | 7162 | 173793 | 7241 | 1220 | 2497 | 223 | 42 | 225 | 8393 | 4484 | 2230 | 1183 | 1009 | 216699 |
| 4 | 5375 | 11645 | 7318 | 133562 | 8894 | 18572 | 1713 | 276 | 238 | 2220 | 608 | 1687 | 1139 | 1322 | 194570 |
| 5 | 4783 | 5366 | 970 | 9034 | 540381 | 139434 | 6839 | 845 | 919 | 371 | 267 | 597 | 3683 | 3487 | 716977 |
| 6 | 6988 | 11317 | 2043 | 18727 | 135542 | 386077 | 18021 | 2526 | 766 | 830 | 1792 | 1129 | 4253 | 4979 | 594990 |
| 7 | 903 | 1678 | 284 | 1771 | 6407 | 15266 | 75801 | 1951 | 2087 | 116 | 12 | 217 | 4589 | 2767 | 113851 |
| 8 | 1992 | 684 | 74 | 560 | 1057 | 2797 | 2313 | 70905 | 1517 | 45 | 22 | 95 | 1026 | 1714 | 84801 |
| 9 | 2654 | 2621 | 132 | 239 | 718 | 656 | 821 | 1632 | 9580 | 129 | 12 | 295 | 507 | 5731 | 25727 |
| 10 | 1126 | 1829 | 7044 | 3550 | 561 | 1153 | 142 | 32 | 118 | 25755 | 7323 | 3537 | 5799 | 615 | 58584 |
| 11 | 400 | 897 | 4623 | 611 | 284 | 2623 | 12 | 34 | 19 | 6070 | 45126 | 13951 | 297 | 461 | 75409 |
| 12 | 646 | 789 | 2199 | 1184 | 373 | 915 | 174 | 61 | 204 | 3239 | 13028 | 248802 | 943 | 1573 | 274130 |
| 13 | 259 | 174 | 546 | 417 | 1830 | 2126 | 3104 | 638 | 341 | 3110 | 153 | 758 | 447 | 1575 | 15478 |
| 14 | 1245 | 3122 | 641 | 862 | 2091 | 2150 | 1935 | 1321 | 3731 | 202 | 309 | 964 | 1557 | 0 | 20130 |
| Total | 428108 | 160752 | 212145 | 193796 | 707340 | 591545 | 112959 | 82577 | 25577 | 53455 | 74250 | 276970 | 27071 | 31041 | 2977584 |

Table 3.6 Inter Peak Hour Prior Matrix Before MVESTM (PCUs x 10)

| OD | 1 | 2 | 3 | 4 | 5 | 6 | 7 | 8 | 9 | 10 | 11 | 12 | 13 | 14 | Total |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | 352042 | 55662 | 5830 | 6513 | 4442 | 6267 | 447 | 1689 | 3247 | 1113 | 425 | 988 | 936 | 2278 | 441878 |
| 2 | 42695 | 57747 | 6647 | 9527 | 3539 | 11013 | 687 | 624 | 2584 | 1702 | 882 | 1518 | 699 | 3529 | 143394 |
| 3 | 6995 | 7162 | 173661 | 7241 | 1220 | 2497 | 185 | 42 | 225 | 8322 | 5104 | 2228 | 1136 | 1009 | 217028 |
| 4 | 5375 | 11645 | 7318 | 133431 | 8894 | 18572 | 861 | 276 | 238 | 2188 | 562 | 1464 | 1187 | 1322 | 193334 |
| 5 | 4783 | 5366 | 970 | 9034 | 540362 | 139434 | 7699 | 845 | 919 | 371 | 298 | 480 | 3629 | 3487 | 717678 |
| 6 | 6988 | 11317 | 2043 | 18727 | 135542 | 386033 | 13166 | 2303 | 766 | 812 | 1804 | 865 | 4269 | 4979 | 589614 |
| 7 | 527 | 793 | 228 | 1053 | 7826 | 12547 | 94003 | 1364 | 168 | 164 | 49 | 263 | 1138 | 1635 | 121757 |
| 8 | 1992 | 684 | 74 | 560 | 1057 | 2324 | 1475 | 70905 | 1517 | 49 | 0 | 76 | 998 | 1714 | 83424 |
| 9 | 2654 | 2621 | 132 | 239 | 718 | 656 | 147 | 1632 | 9580 | 119 | 23 | 297 | 274 | 5731 | 24821 |
| 10 | 1103 | 1757 | 6782 | 3515 | 565 | 1144 | 166 | 41 | 111 | 26553 | 5565 | 2615 | 6708 | 636 | 57261 |
| 11 | 414 | 852 | 5128 | 540 | 321 | 2656 | 48 | 0 | 27 | 5141 | 48761 | 15307 | 371 | 468 | 80033 |
| 12 | 640 | 741 | 2163 | 1148 | 269 | 647 | 231 | 56 | 203 | 2704 | 14590 | 249479 | 942 | 1475 | 275287 |
| 13 | 283 | 246 | 595 | 428 | 1820 | 2155 | 984 | 631 | 200 | 4328 | 285 | 758 | 422 | 1528 | 14663 |
| 14 | 1245 | 3122 | 641 | 862 | 2091 | 2150 | 1455 | 1321 | 3731 | 233 | 351 | 899 | 1556 | 0 | 19657 |
| Total | 427734 | 159715 | 212211 | 192818 | 708667 | 588094 | 121555 | 81728 | 23516 | 53800 | 78699 | 277237 | 24263 | 29792 | 2979829 |

Table 3.7 Inter Peak Hour TMfS:05A Final Matrix (PCUs x 10)

| OD | 1 | 2 | 3 | 4 | 5 | 6 | 7 | 8 | 9 | 10 | 11 | 12 | 13 | 14 | Total |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | 354669 | 51508 | 5184 | 5935 | 3995 | 5410 | 381 | 1651 | 2935 | 967 | 328 | 791 | 622 | 2097 | 436472 |
| 2 | 42440 | 56323 | 6492 | 9379 | 3459 | 10327 | 587 | 602 | 2493 | 1616 | 779 | 1271 | 523 | 3173 | 139464 |
| 3 | 6314 | 5472 | 174605 | 7346 | 981 | 2085 | 121 | 25 | 184 | 8303 | 4899 | 2056 | 1265 | 675 | 214332 |
| 4 | 4663 | 9615 | 7156 | 134379 | 8614 | 18225 | 755 | 206 | 192 | 2206 | 577 | 1292 | 903 | 1101 | 189884 |
| 5 | 4499 | 5079 | 855 | 8687 | 545826 | 139313 | 6955 | 631 | 864 | 324 | 251 | 295 | 3012 | 2728 | 719320 |
| 6 | 6536 | 10615 | 1819 | 18175 | 134993 | 387874 | 14571 | 1906 | 764 | 734 | 1627 | 668 | 4803 | 4359 | 589443 |
| 7 | 513 | 778 | 182 | 966 | 7129 | 14368 | 100259 | 1392 | 189 | 139 | 39 | 187 | 1298 | 1608 | 129045 |
| 8 | 2032 | 616 | 66 | 507 | 987 | 2282 | 1644 | 71088 | 1648 | 40 | 0 | 52 | 1061 | 1836 | 83858 |
| 9 | 2506 | 2620 | 118 | 235 | 722 | 649 | 147 | 1545 | 10946 | 107 | 19 | 184 | 156 | 5829 | 25782 |
| 10 | 1054 | 1277 | 6883 | 3336 | 528 | 1008 | 127 | 27 | 90 | 26454 | 6894 | 2739 | 5839 | 481 | 56738 |
| 11 | 321 | 500 | 4955 | 474 | 238 | 2001 | 33 | 0 | 21 | 5643 | 49514 | 15115 | 396 | 317 | 79528 |
| 12 | 544 | 483 | 2176 | 941 | 205 | 567 | 172 | 37 | 144 | 2744 | 14536 | 242895 | 1039 | 1033 | 267515 |
| 13 | 216 | 183 | 572 | 330 | 1791 | 2803 | 1689 | 1113 | 136 | 3913 | 303 | 821 | 353 | 1660 | 15882 |
| 14 | 1207 | 2946 | 579 | 785 | 2110 | 2626 | 1553 | 1402 | 3858 | 201 | 292 | 632 | 1577 | 0 | 19769 |
| Total | 427515 | 148016 | 211641 | 191473 | 711578 | 589537 | 128993 | 81624 | 24465 | 53391 | 80058 | 268997 | 22848 | 26895 | 2967033 |

Table 3.8 PM Peak Hour TMfS:05 Matrix (PCUs x 10)

| OD | 1 | 2 | 3 | 4 | 5 | 6 | 7 | 8 | 9 | 10 | 11 | 12 | 13 | 14 | Total |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | 479404 | 93335 | 16948 | 13539 | 3324 | 10436 | 1584 | 2790 | 11309 | 5828 | 1082 | 736 | 679 | 2335 | 643328 |
| 2 | 55807 | 90946 | 10328 | 18505 | 5267 | 17966 | 1099 | 576 | 2960 | 2670 | 388 | 1096 | 338 | 6118 | 214064 |
| 3 | 9597 | 6613 | 249767 | 8469 | 1380 | 3232 | 96 | 31 | 192 | 12934 | 5702 | 4195 | 1375 | 497 | 304080 |
| 4 | 6877 | 16337 | 9095 | 204540 | 13274 | 28735 | 2859 | 185 | 369 | 4627 | 316 | 1205 | 1291 | 1505 | 291215 |
| 5 | 6049 | 7781 | 1038 | 18815 | 687643 | 235468 | 16525 | 1084 | 3396 | 1428 | 333 | 1377 | 9299 | 4397 | 994633 |
| 6 | 7896 | 17985 | 1379 | 25376 | 168409 | 532535 | 36096 | 3811 | 1041 | 1036 | 236 | 563 | 5212 | 3577 | 805152 |
| 7 | 450 | 1105 | 93 | 1933 | 10048 | 25861 | 94425 | 2457 | 545 | 150 | 2 | 197 | 8593 | 2481 | 148340 |
| 8 | 727 | 1394 | 56 | 250 | 1757 | 3234 | 2274 | 82347 | 1284 | 25 | 116 | 46 | 529 | 3103 | 97143 |
| 9 | 2964 | 3405 | 143 | 159 | 2656 | 631 | 3095 | 1785 | 12429 | 36 | 1 | 8 | 125 | 5992 | 33428 |
| 10 | 5088 | 2123 | 12391 | 4485 | 652 | 1549 | 214 | 13 | 231 | 14408 | 8803 | 5070 | 6266 | 1104 | 62398 |
| 11 | 246 | 272 | 10223 | 641 | 518 | 1155 | 9 | 1 | 2 | 9235 | 68029 | 23983 | 976 | 47 | 115337 |
| 12 | 595 | 1248 | 2485 | 3316 | 241 | 1188 | 291 | 18 | 316 | 5575 | 14281 | 356871 | 1707 | 850 | 388981 |
| 13 | 163 | 94 | 1113 | 483 | 2322 | 2716 | 4589 | 290 | 19 | 3306 | 95 | 1339 | 695 | 768 | 17992 |
| 14 | 1382 | 2730 | 306 | 755 | 2681 | 2348 | 2040 | 1313 | 4246 | 817 | 474 | 1421 | 651 | 0 | 21164 |
| Total | 577245 | 245368 | 315366 | 301263 | 900171 | 867054 | 165196 | 96701 | 38339 | 62075 | 99859 | 398108 | 37735 | 32776 | 4137256 |

Table 3.9 PM Peak Hour Prior Matrix Before MVESTM (PCUs x 10)

| OD | 1 | 2 | 3 | 4 | 5 | 6 | 7 | 8 | 9 | 10 | 11 | 12 | 13 | 14 | Total |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | 479370 | 93335 | 16948 | 13539 | 3324 | 10436 | 996 | 2790 | 11309 | 5790 | 1121 | 686 | 664 | 2335 | 642642 |
| 2 | 55807 | 90678 | 10328 | 18505 | 5267 | 17966 | 1002 | 576 | 2960 | 2495 | 341 | 1075 | 360 | 6118 | 213476 |
| 3 | 9597 | 6613 | 249377 | 8469 | 1380 | 3232 | 311 | 31 | 192 | 12895 | 5521 | 4242 | 1282 | 497 | 303640 |
| 4 | 6877 | 16337 | 9095 | 204123 | 13274 | 28735 | 1490 | 185 | 369 | 4551 | 235 | 1117 | 1344 | 1505 | 289239 |
| 5 | 6049 | 7781 | 1038 | 18815 | 687486 | 235468 | 17921 | 1084 | 3396 | 1428 | 153 | 938 | 9233 | 4397 | 995186 |
| 6 | 7896 | 17985 | 1379 | 25376 | 168409 | 531977 | 22567 | 3764 | 1041 | 1014 | 205 | 316 | 5204 | 3577 | 790710 |
| 7 | 362 | 795 | 315 | 1087 | 10560 | 17904 | 159432 | 1793 | 115 | 239 | 26 | 41 | 1575 | 972 | 195216 |
| 8 | 727 | 1394 | 56 | 250 | 1757 | 3197 | 1509 | 82347 | 1284 | 24 | 116 | 31 | 420 | 3103 | 96217 |
| 9 | 2964 | 3405 | 143 | 159 | 2656 | 631 | 423 | 1785 | 12429 | 36 | 1 | 5 | 110 | 5992 | 30737 |
| 10 | 5094 | 2095 | 12564 | 4512 | 642 | 1587 | 182 | 13 | 231 | 16656 | 5529 | 3614 | 8767 | 1152 | 62638 |
| 11 | 368 | 368 | 11682 | 477 | 398 | 983 | 23 | 0 | 23 | 8877 | 58205 | 24678 | 1143 | 69 | 107294 |
| 12 | 409 | 1242 | 2577 | 3383 | 250 | 810 | 269 | 15 | 308 | 4958 | 13829 | 357824 | 1705 | 750 | 388329 |
| 13 | 169 | 101 | 1193 | 528 | 2318 | 2698 | 1717 | 229 | 28 | 4428 | 148 | 1213 | 429 | 773 | 15972 |
| 14 | 1382 | 2730 | 306 | 755 | 2681 | 2348 | 1086 | 1313 | 4246 | 852 | 507 | 1388 | 401 | 0 | 19997 |
| Total | 577070 | 244860 | 317002 | 299977 | 900400 | 857974 | 208929 | 95926 | 37931 | 64242 | 85937 | 397167 | 32637 | 31241 | 4151294 |

Table 3.10 PM Peak Hour TMfS:05A Final Matrix (PCUs x 10)

| OD | 1 | 2 | 3 | 4 | 5 | 6 | 7 | 8 | 9 | 10 | 11 | 12 | 13 | 14 | Total |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | 481603 | 89692 | 15729 | 11546 | 2842 | 9179 | 909 | 2604 | 10747 | 5611 | 1035 | 611 | 489 | 2200 | 634795 |
| 2 | 54182 | 89189 | 9242 | 18503 | 4361 | 16496 | 835 | 509 | 2760 | 2441 | 316 | 805 | 239 | 5320 | 205197 |
| 3 | 8447 | 5845 | 249440 | 8932 | 1085 | 3055 | 197 | 23 | 164 | 12963 | 5341 | 4247 | 1462 | 397 | 301598 |
| 4 | 6057 | 14830 | 8752 | 206736 | 12236 | 28095 | 1482 | 165 | 330 | 4658 | 295 | 1197 | 1952 | 1319 | 288104 |
| 5 | 6065 | 8083 | 775 | 17276 | 700720 | 231342 | 14928 | 807 | 3236 | 1271 | 184 | 756 | 8232 | 3423 | 997098 |
| 6 | 7858 | 18655 | 1042 | 23959 | 164865 | 533977 | 23624 | 3100 | 1059 | 968 | 233 | 287 | 5297 | 3529 | 788454 |
| 7 | 342 | 859 | 190 | 934 | 9224 | 24930 | 157548 | 1971 | 122 | 185 | 22 | 30 | 3049 | 868 | 200275 |
| 8 | 698 | 1469 | 47 | 232 | 1495 | 3213 | 1603 | 82069 | 1235 | 20 | 75 | 28 | 476 | 3342 | 96001 |
| 9 | 2999 | 2975 | 93 | 124 | 2650 | 641 | 444 | 1606 | 13447 | 22 | 0 | 4 | 48 | 6336 | 31389 |
| 10 | 5096 | 1852 | 13091 | 4636 | 597 | 1599 | 175 | 10 | 214 | 15844 | 7503 | 3777 | 7995 | 951 | 63341 |
| 11 | 294 | 259 | 10220 | 430 | 357 | 900 | 34 | 0 | 17 | 9446 | 63230 | 24519 | 1268 | 47 | 111020 |
| 12 | 283 | 742 | 2207 | 2888 | 199 | 644 | 156 | 9 | 187 | 4366 | 14530 | 342220 | 1716 | 498 | 370645 |
| 13 | 113 | 79 | 1336 | 412 | 1982 | 3328 | 2336 | 257 | 19 | 3969 | 213 | 1507 | 403 | 873 | 16827 |
| 14 | 1457 | 2576 | 247 | 717 | 2648 | 2402 | 1474 | 1428 | 4423 | 756 | 625 | 1279 | 693 | 0 | 20725 |
| Total | 575495 | 237106 | 312413 | 297325 | 905261 | 859801 | 205746 | 94558 | 37961 | 62520 | 93602 | 381264 | 33318 | 29101 | 4125471 |

1: Edinburgh
2: Lothian
3: Fife
4: Central
5: Glasgow
6: Strathclyde
7: Ayrshire
8: Dumfries \& Galloway
9: Borders
10: Perthshire
11: Dundee
12: North East
13: External (North)
14: External (South)

Figure 3.1 14 Sector Definition


Figure 3.2 Zone Split


Figure 3.3 TMfS:05A Zoning System


Figure 3.4 TMfS:05A RSI Sites

## 4 Assignment Model Development

### 4.1 Introduction

4.1.1 The assignment procedure adopted for TMfS:05A HAM is the same as that used in TMfS.05, namely a 'Volume Averaged Capacity Restraint Assignment' based on 'All or Nothing' paths at each iteration (for four user classes).
4.1.2 The TMfS:05A HAM includes:

■ four separate user classes are assigned to the network. These are; Car In Work, Car Non Work, LGV and OGV; and

- the assignment adopts the principles of the 'Davis Method', which allows for modelling of tolls to be undertaken during the main assignment rather than as a separated modelling process.
4.1.3 This chapter describes assignment procedure used for TMfS:05A HAM plus the incorporation of the 'Cost versus Time' Assignment Method.


### 4.2 Assignment procedure

4.2.1 The assignment procedure adopted is a 'Volume Averaged Capacity Restraint Assignment' based on 'All or Nothing' paths at each iteration. This procedure has the following benefits:

- model convergence can be checked;
- the assignment can continue for as many iterations as required to achieve a user predefined level of convergence;
- cars, goods and light vehicles are assigned using the same path building technique on every iteration; and
- 'All or Nothing' path building at each iteration gives a comprehensive multi routing assignment.
4.2.2 The assignment procedure carries out a 'Volume Averaged Capacity Restraint' throughout the whole modelled area, based on 'All or Nothing' paths for ' $n$ ' iterations until the model is fully converged. The principal features of this assignment process are as follows:
- the model operates over three one hour time periods;
- 'All or Nothing' path building is carried out separately for the four user classes (car in work, car non work, LGV and OGV) using the CUBE program AVROAD; and
- 'Volume Averaged Capacity Restraint' (within the CUBE program AVCAP) ensures that each iteration of restraint is based on the average of all previous iterations (during capacity restraint calculations, all user classes are combined into total PCUs).
4.2.3 'Volume Averaged Capacity Restraint' is ideally suited to congested urban networks, where the level of traffic leads to different 'All or Nothing' paths on successive iterations, and so to multi-routing through the 'Volume Averaging' procedure. However, an uncongested rural area will tend to give mono routing results because of the low level of traffic compared with
capacity and the reduced routing choices. As a result, the optimum paths on the first iteration will remain the optimum throughout the assignment.


### 4.3 Cost versus Time Assignment Method

4.3.1 The 'Cost versus Time Assignment Method' (CvT Method) was incorporated within the previous versions of the TMfS assignment procedure as it allows tolling tests to be undertaken without the requirement of a separate model, as was the case in both CSTM models ( 3 and 3A). This is still the case within TMfS:05A.
4.3.2 The method is described in the paper entitled "Cost versus Time Equilibrium over a Network" by Fabien Leurent in the "European Journal of Operational Research". The paper describes the theory and demonstrates that the method converges to equilibrium.
4.3.3 Rather than increase the number of user classes, this method varies the willingness to pay weighting applied to tolls in the route choice generalised cost from iteration to iteration. The willingness to pay weighting is in fact randomly sampled from a distribution, which is representative of the total population. The mechanics of the process are very similar to the stochastic user equilibrium process.
4.3.4 The generalised cost for route choice is defined for a link in the network as:
$\mathrm{C}=\mathrm{a} *$ time $+\mathrm{b} *$ distance $+\mathrm{c} *$ toll
4.3.5 In the equation above, ' $a$ ' is a time parameter, ' $b$ ' $a$ distance parameter and ' $c$ ' $a$ cost parameter.
4.3.6 Where ' $C$ ' is the link generalised cost and ' $a$ ', ' $b$ ' and ' $c$ ' are parameters. In the CSTM, tolling model there was one value of ' $c$ ' for each user class (for a particular year) and these values are fixed for the whole assignment. In the CVT method, there are no additional user classes compared with the standard (ie non-toll) model but the parameter ' $c^{\prime}$ (one for each user class) is varied by random sampling at each iteration of the highway assignment procedure.
4.3.7 The distributions, from which the 'willingness to pay' for each user class are randomly sampled, remain the same in TMfS:05A.

### 4.4 Model Convergence

4.4.1 The methodology for calculating model convergence in the TMfS:05A HAM is the same as that for TMfS. 05 .
4.4.2 From the iteration number and the total cost, a normalised regression statistic is calculated using the following formula (which provides the gradient of the line of the graph of iteration number ' $X$ ' versus total cost).
$a=\frac{n \sum x y-\sum x \sum y}{n \sum(x)^{2}-\left(\sum x\right)^{2}} / c$
where:
a is the gradient;
x is the iteration number;
y is the total cost;
c is the total cost on the current iteration; and
n is the number of iterations over which the regression is calculated.
4.4.3 The regression statistic is normalised using the total cost of the current iteration, to leave it unitless as a pure parameter.
4.4.4 The HAM acceptance criteria is that the level of convergence must be less than or equal to the DMRB recommended value of $1 \%$ on three successive iterations for the assignment procedure to automatically terminate. This is a very exacting level of convergence for this size of model and is necessary to ensure that reliable data is passed to other elements of the modelling process, most importantly, the economic analysis element.
4.4.5 The number of iterations required to reach convergence within the base model were (TMfS:05 values in brackets):

■ AM Peak - 68 iterations (77);
■ Inter-Peak - 30 iterations (36); and
■ PM Peak - 67 iterations (68).

### 5.1 Introduction

5.1.1 In this chapter, the model is examined in detail to demonstrate its level of calibration. Journey time validation and validation against independent counts are presented in the following chapter. All observed and modelled values are in vehicles.
5.1.2 The screenline locations and traffic count data used for calibration purposes are those used in the MVESTM process. In total, 739 sites have been used in the MVESTM procedure in the morning and evening peaks and 723 in the inter peak (when the Kilmarnock RSIs were not undertaken). Of these 739/723 sites, 104 formed part of multi-point screenlines and, as such, are duplicates. Therefore, the number of unique screenlines is 635 in the morning and evening peaks and 619 in the inter peak.
5.1.3 The analysis of the modelled screenline and link flows makes use of a summary statistic known as GEH, which is defined as:

$$
G E H=\left(\frac{(\text { observed }- \text { modelled })^{2}}{(0.5 \times(\text { observed }+ \text { modelled })}\right)^{0.5}
$$

5.1.4 The GEH value is designed to be more tolerant of large percentage differences at lower flows. For example, one would not normally be concerned about a modelled flow that differed from a count by $40 \%$ if the count was only 100 , but one would if the count were 1000 . The reason for introducing such a statistic is the inability of either the absolute difference or the relative difference between the modelled flow and count to reflect differences over a wide range of flows such as are present in the HAM.
5.1.5 For a model such as the HAM, given its size, complexity, and the magnitude of traffic flows, we would normally expect screenline GEH values to meet the following targets to achieve a high standard of calibration:

| - | $\mathrm{GEH}<5$ | $60 \%$ of all sites; |
| :--- | :--- | :--- |
| - | $\mathrm{GEH}<7$ | $80 \%$ of all sites; |
| - | $\mathrm{GEH}<10$ | $95 \%$ of all sites; and |
| $\mathrm{GEH}<12$ | $100 \%$ of all sites. |  |

### 5.2 Key Strategic Screenline Flows

5.2.1 Key strategic screenlines are defined for the purposes of model calibration, as shown in Appendix B. One strategic screenline covers traffic flows across the Forth Estuary on the Kincardine Bridge, the Forth Road Bridge and at Stirling (calibration screenlines 17 and 117). The results for TMfS:05A are presented in Table 5.1 and the results for TMfS:05 are detailed in Table 5.2 for comparison purposes.

Table 5.1 TMfS:05A Forth Estuary Strategic Screenline

| Direction | Time <br> Period | Observed | Modelled | Dif | \%Dif | GEH |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Northbound | AM | 4696 | 4715 | 19 | 0.4 | 0.3 |
|  | IP | 3446 | 3442 | -4 | -0.1 | 0.1 |
| Southbound | PM | 6096 | 5759 | -337 | -5.5 | 4.4 |
|  | AM | 5072 | 5030 | -42 | -0.8 | 0.6 |
|  | IP | 3312 | 3535 | 223 | 6.7 | 3.8 |
|  | PM | 4387 | 4462 | 75 | 1.7 | 1.1 |

Table 5.2 TMfS:05 Forth Estuary Strategic Screenline

| Direction | Time <br> Period | Observed | Modelled | Dif | \%Dif | GEH |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Northbound | AM | 4696 | 4652 | -44 | -0.9 | 0.6 |
|  | IP | 3446 | 3456 | 10 | 0.3 | 0.2 |
| Southbound | PM | 6096 | 5694 | -402 | -6.6 | 5.2 |
|  | AM | 5072 | 5179 | 107 | 2.1 | 1.5 |
|  | IP | 3312 | 3606 | 294 | 8.9 | 5.0 |
|  | PM | 4387 | 4530 | 143 | 3.3 | 2.1 |

5.2.2 TMfS:05A Northbound screenlines show that observed and modelled flows differ by between a GEH of 0.1 and 4.4 and the Southbound screenline differs by between a GEH of 0.6 and 3.8. Comparing TMfS:05A GEHs with those of TMfS:05 GEHs, both the Northbound and Southbound directions demonstrate an improvement in all time periods.
5.2.3 The strategic screenline across the River Clyde includes all crossings from the Albert Bridge, east of Glasgow City Centre, to the Erskine Bridge in the west (calibration screenlines 246 and 346). Table 5.3 presents TMfS:05A observed versus modelled flows for this screenline while Table 5.4 presents TMfS:05 screenline data.

Table 5.3 TMfS:05A Clyde Strategic Screenline

| Direction | Time <br> Period | Observed | Modelled | Dif | \%Dif | GEH |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Northbound | AM | 15426 | 14954 | -472 | -3.1 | 3.8 |
|  | IP | 9807 | 10039 | 232 | 2.4 | 2.3 |
| Southbound | PM | 11208 | 10898 | -310 | -2.8 | 3.0 |
|  | AM | 12386 | 13721 | 1335 | 10.8 | 11.7 |
|  | IP | 10183 | 10918 | 735 | 7.2 | 7.2 |
|  | PM | 15848 | 16941 | 1093 | 6.9 | 8.5 |

Table 5.4 TMfS:05 Clyde Strategic Screenline

| Direction | Time <br> Period | Observed | Modelled | Dif | \%Dif | GEH |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Northbound | AM | 15426 | 15137 | -289 | -1.9 | 2.3 |
|  | IP | 9807 | 10130 | 323 | 3.3 | 3.2 |
| Southbound | PM | 11208 | 11433 | 225 | 2.0 | 2.1 |
|  | AM | 12386 | 13816 | 1430 | 11.6 | 12.5 |
|  | IP | 10183 | 10679 | 496 | 4.8 | 4.9 |
|  | PM | 15848 | 16480 | 632 | 4.0 | 5.0 |

5.2.4 TMfS:05A Northbound screenlines show that observed and modelled flows differ by between a GEH of 2.3 and 3.8 and the Southbound screenline differs by between a GEH of 7.2 and 11.7. Comparison of the tables shows some changes in the level of calibration and this is principally due to the update of demand matrices, and particularly the matrix estimation process.
5.2.5 Table 5.5 presents TMfS:05A results for Strategic Screenline Three, which covers traffic flows across the Tay Bridge (calibration screenlines 27 and 127). Table 5.6 presents the results for TMfS:05 for comparison.

Table 5.5 TMfS:05A Tay Strategic Screenline

| Direction | Time <br> Period | Observed | Modelled | Dif | \%Dif | GEH |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Northbound | AM | 1957 | 2117 | 160 | 8.2 | 3.6 |
|  | IP | 725 | 811 | 86 | 11.8 | 3.1 |
| Southbound | PM | 909 | 980 | 71 | 7.9 | 2.3 |
|  | AM | 718 | 866 | 148 | 20.6 | 5.3 |
|  | IP | 722 | 767 | 45 | 6.2 | 1.6 |
|  | PM | 1442 | 1376 | -66 | -4.6 | 1.8 |

Table 5.6 TMfS:05 Tay Strategic Screenline

| Direction | Time <br> Period | Observed | Modelled | Dif | \%Dif | GEH |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Northbound | AM | 1957 | 1793 | -164 | -8.4 | 3.8 |
|  | IP | 725 | 745 | 20 | 2.8 | 0.7 |
| Southbound | PM | 909 | 991 | 82 | 9.0 | 2.7 |
|  | AM | 718 | 837 | 119 | 16.6 | 4.3 |
|  | IP | 722 | 716 | -6 | -0.8 | 0.2 |
|  | PM | 1442 | 1292 | -150 | -10.4 | 4.1 |

5.2.6 TMfS:05A Northbound screenlines show that observed and modelled flows differ by between a GEH of 2.3 and 3.6 and the Southbound screenlines differ between a GEH of 1.6 and 5.3. Comparison of the tables shows some changes in the level of calibration and this is principally due to the update of demand matrices, and particularly the matrix estimation process. In general the level of calibration is considered to be broadly similar and of a good standard. It should be noted that the observed counts shown in the tables are the same data as that used for TMfS:05 and not the new RSI count used during matrix estimation, which are included in Appendix D.

### 5.3 Other screenline flows

5.3.1 As discussed in paragraph 5.1.2, the calibration screenlines presented in this chapter are the same as those used in the MVESTM process (Appendix B).
5.3.2 Given that the principal aim of this project is to predict strategic road flows throughout the modelled area, the calibration sites can be conveniently divided into two groups:

- key links (single points on major roads); and
- multi-point screenlines.


### 5.4 Key Links

5.4.1 The same key links as TMfS:05 have been used when calibrating TMfS:05A. The links presented here may also exist as part of multi-point screenlines but are presented separately, given the importance of these routes to the objectives of the model. Appendix $C$ presents tables for the AM peak, Inter-Peak and PM peak observed/modelled total flows for the 253 Key Links, which have been used to achieve calibration throughout the HAM. The GEH statistic (described in paragraph 5.1.4) has been used to assess the overall acceptability of these results. For ease of comparison between the TMfS:05 and the TMfS:05A results, the TMfS:05 results are contained in brackets in all tables.
5.4.2 These Key Links cover the major roads of the modelled area. Table 5.7 details the GEH analysis.

Table 5.7 Key Links Flow GEH Analysis

| \% of sites with GEH value (TMfS:05 values in brackets) |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: |
| Time | $\leq 5$ | $\leq 7$ | $\leq 10$ | $\leq 12$ | $\leq 15$ |
| Period |  |  |  |  |  |
| AM | $68(62)$ | $81(78)$ | $93(89)$ | $97(95)$ | $100(100)$ |
| IP | $80(79)$ | $91(90)$ | $98(96)$ | $99(99)$ | $100(100)$ |
| PM | $66(66)$ | $83(79)$ | $94(91)$ | $100(95)$ | $100(99)$ |
| Target | $60 \%$ | $80 \%$ | $95 \%$ | $100 \%$ |  |

5.4.3 The vast majority of the GEH values are better than the target of 12 , indicating that the major routes of the modelled area are sufficiently well calibrated. The GEH percentages compare favourably against those of TMfS:05 results. In this instance, the number of sites that have GEH values of $\leq 5, \leq 7$, and $\leq 10$ for all time periods are greater than or equal to the TMfS:05 values.
5.4.4 The highest GEH statistics are 16.14 in the AM Peak; 13.91 in the Inter-Peak; and 16.04 in the PM Peak.

### 5.5 Multi-Point Screenlines

5.5.1 In addition to single link calibration points, a number of screenlines with multiple observations were prepared. These multi-point screenlines were used to calibrate the model across a cordon or along a wide screenline. The same screenlines as TMfS:05 have been used when calibrating TMfS:05A with addition of new RSI count data. Appendix D provides a detailed analysis of these multi-point screenline flows.
5.5.2 Table 5.8 summarises the screenline GEH analysis for each time period for the multi-point screenlines used in the calibration of the model. In general, the screenline GEH value is better than the target of 12 . Table 5.8 shows that the majority of GEH values lay within or close to their target levels.

## Table 5.8 Multi Point Screenlines GEH Analysis

| \% of sites with GEH value (TMfS:05 values in brackets) |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: |
| Time | $\leq 5$ | $\leq 7$ | $\leq 10$ | $\leq 12$ | $\leq 15$ |
| Period |  |  |  |  |  |
| AM | $60(62)$ | $74(77)$ | $87(91)$ | $93(97)$ | $97(100)$ |
| IP | $67(67)$ | $83(71)$ | $95(94)$ | $98(99)$ | $100(100)$ |
| PM | $60(62)$ | $75(75)$ | $88(90)$ | $94(96)$ | $97(99)$ |
| Target | $60 \%$ | $80 \%$ | $95 \%$ | $100 \%$ |  |

5.5.3 Appendix $E$ provides a detailed breakdown of the flows on each individual link used in calibrating the HAM for all three time periods. Table 5.9 summarises the GEH analysis of these $635 / 619$ sites. Appendix $E$ also shows these GEH values as coloured links on the network for each modelled time period.

Table 5.9 Link Flows - Calibration Screenlines

| \% of sites with GEH value (TMfS:05 values in brackets) |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Time Period | $\leq 5$ | $\leq 7$ | $\leq 10$ | $\leq 12$ | $\leq 15$ |
| AM | 62 (60) | 74 (74) | 88 (88) | 94 (94) | 98 (99) |
| IP | 73 (72) | 86 (84) | 96 (95) | 98 (99) | 100 (100) |
| PM | 61 (62) | 76 (76) | 88 (89) | 95 (93) | 98 (98) |
| Target | 60\% | 80\% | 95\% | 100\% |  |

5.5.4 A large number of the links in the TMfS:05A network are within the GEH target of 12 , and the vast majority are better than the target of 15 . However, the highest GEH values are 21.64 in the AM peak, 15.91 in the Inter-Peak and 26.74 in the PM peak respectively. These sites were investigated and the difficulties lie in the relative coarseness and large size of zones in the vicinity and corresponding lack of assigned intra-zonal trips, which would increase traffic on these links. Of all the Key Links with a GEH value greater than 15, 12 are in the AM peak, 1 in the Inter-Peak and 14 in the PM peak.
5.5.5 The TMfS:05A values are similar to those of the TMfS: 05 values, with changes being relatively marginal.
5.5.6 Appendix $F$ contains graphical illustrations of the screenline results for the three time periods.

## 6 Validation

### 6.1 Introduction

6.1.1 Validation is the process of checking how well the model compares with data independent of the calibration process and will be presented using the following information:

- journey time data;
- count data not used in calibration; and
- trip length distribution analysis.
6.1.2 In addition, screenline analysis has been undertaken on HGVs. This analysis was not used during calibration as the calibration process considered flows in terms of total vehicles only.


### 6.2 STPR Journey Time Survey Routes

6.2.1 As part of the validation process, observed and modelled journey times have been compared across twenty routes. This journey time data was collected as part of the Strategic Transport Projects Review (STPR) and was also used to determine the capacity indices and link speeds as described in Chapter 2 of this report. Table 6.1 illustrates the journey time comparisons.

Table 6.1 STPR Journey Time Routes

| Route | Dir | $\begin{array}{r} \text { Obs } \\ \text { (mins) } \end{array}$ | $\begin{array}{r} \text { Mod } \\ \text { (mins) } \end{array}$ | Within DMRB Criteria |
| :---: | :---: | :---: | :---: | :---: |
| Route 1 Inverness To Elgin | WB | 47.7 | 46.7 | Yes |
|  | EB | 48.4 | 47.6 | Yes |
| Route 2 Elgin to Aberdeen | EB | 79.2 | 79.8 | Yes |
|  | WB | 82.2 | 80.8 | Yes |
| Route 3 Inverness to Aviemore | NB | 29.9 | 30.0 | Yes |
|  | SB | 39.5 | 39.2 | Yes |
| Route 4 Ullapool to Inverness | NB | 65.3 | 65.7 | Yes |
|  | SB | 70.8 | 70.0 | Yes |
| Route 5 Inverness to Dornoch | NB | 48.4 | 47.5 | Yes |
|  | SB | 50.4 | 50.2 | Yes |
| Route 6 Dornoch to Helmsdale | NB | 35.0 | 34.9 | Yes |
|  | SB | 33.6 | 33.3 | Yes |
| Route 7 Helmsdale to Thurso | NB | 47.3 | 48.1 | Yes |
|  | SB | 61.4 | 61.0 | Yes |
| Route 8 Thurso to Latheron | NB | 44.6 | 44.0 | Yes |
|  | SB | 49.6 | 48.7 | Yes |


| Route | Dir | $\begin{array}{r} \text { Obs } \\ \text { (mins) } \end{array}$ | $\begin{array}{r} \text { Mod } \\ \text { (mins) } \end{array}$ | Within DMRB Criteria |
| :---: | :---: | :---: | :---: | :---: |
| Route 10 Invergarry to Kyle of Lochalsh | WB | 63.3 | 62.3 | Yes |
|  | EB | 62.9 | 61.9 | Yes |
| Route 15 Inverness to Fort William | NB | 92.7 | 93.1 | Yes |
|  | SB | 96.3 | 96.2 | Yes |
| Route 16 Crianlarich to Oban | WB | 47.0 | 46.7 | Yes |
|  | EB | 47.7 | 47.2 | Yes |
| Route 17 Crainlarich to Fort William | NB | 65.8 | 65.8 | Yes |
|  | SB | 70.2 | 68.8 | Yes |
| Route 18 Fort William to Mallaig | WB | 54.9 | 54.2 | Yes |
|  | EB | 53.7 | 52.0 | Yes |
| Route 20 Dunkeld to Aviemore | NB | 70.2 | 70.6 | Yes |
|  | SB | 74.1 | 73.7 | Yes |
| Route 23 Tarbet to Cambeltown | SB | 129.8 | 129.6 | Yes |
|  | NB | 128.1 | 126.8 | Yes |
| Route 26 Aviemore to Keith | WB | 64.0 | 63.2 | Yes |
|  | EB | 82.6 | 78.9 | Yes |
| Route A Perth to Dunkeld | NB | 11.0 | 11.1 | Yes |
|  | SB | 12.1 | 12.2 | Yes |
| Route B - Alexandra to Crianlarich | NB | 50.9 | 49.6 | Yes |
|  | SB | 50.3 | 49.1 | Yes |
| Route C Invermoriston to A887-A87 junction | WB | 17.5 | 17.3 | Yes |
|  | EB | 17.5 | 17.3 | Yes |
| Route D Oban to Ballachulish | NB | 33.2 | 32.8 | Yes |
|  | SB | 38.9 | 39.8 | Yes |

6.2.2 Table 6.1 shows that all the modelled journey times fall within the DMRB criteria, ie they are within $15 \%$ of the mean observed journey or one minute if higher.
6.2.3 Table 6.1 shows that the nearly all the modelled journey times fall within the confidence level, ie within upper and lower $95 \%$ confidence intervals. Only one route (Route 8 northbound) is outwith these confidence intervals, however, the GEH indicator is less than 1. Closer inspection of the journey time route indicates that the confidence intervals are very small at $+/-17$ seconds for a total average journey time in excess of 44 minutes. The modelled journey is only 33 seconds below the observed value and, therefore, is considered to be a good match.

### 6.3 Journey Time Validation

6.3.1 As part of the validation process, observed and modelled journey times have been compared across 59 routes throughout the modelled area. These are the same routes used to validate TMfS. 05 .
6.3.2 Table 6.2 and Figure 6.2 (at the end of this chapter) detail the 'Edinburgh Area Urban Journey Routes'. Each individual route is illustrated in Appendix G. Table 6.2 also shows the mean observed and modelled journey times for each route in each time period.

Table 6.2 Edinburgh Area Urban Journey Routes

| Route | Dir | Description | Obs | Mod | AM <br> Within <br> DMRB <br> Criteria | Obs | Mod | IP <br> Within DMRB Criteria | Obs | Mod | PM <br> Within <br> DMRB <br> Criteria |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| B1 | 1 | A720 Lothianburn Junction to B701 Wester Hailes Road / Harvesters Way | 34.6 | 43.5 | No | 30.3 | 35.7 | No | 38.2 | 42.8 | Yes |
|  | 2 | B701 Wester Hailes Road / Harvesters Way to A720 Lothianburn Junction | 35.3 | 46.3 | No | 41.2 | 33.4 | No | 30.1 | 46.2 | No |
| B2 | 1 | A989 Tay St / A85 West of <br> Bridge to A90 / A929 / A972 <br> Dumbbell Roundabout | 33.9 | 27.1 | No | 30.0 | 26.9 | Yes | 37.3 | 27.5 | No |
|  | 2 | A90 / A929 / A972 Dumbbell Roundabout to A989 Tay St / A85 West of Bridge | 25.8 | 27.3 | Yes | 25.9 | 26.4 | Yes | 26.5 | 26.9 | Yes |
| B8 | 1 | M9 J3 Off Slip / A803 to A6095 <br> Dumbbell Roundabout A1 Slips | 58.5 | 53.5 | Yes | 45.2 | 44.9 | Yes | 49.5 | 52.6 | Yes |
|  | 2 | A6095 Dumbbell Roundabout A1 Slips to M9 J3 On Slip / A803 | 57.2 | 53.1 | Yes | 41.1 | 43.9 | Yes | 79.2 | 58.1 | No |
| B11 | 1 | A8 Glasgow Rd / Maybury Rd to A71 / A720 City Bypass | 31.3 | 41.4 | No | 25.0 | 27.6 | Yes | 34.0 | 36.1 | Yes |
|  | 2 | A71 / A720 City Bypass to A8 Glasgow Rd / Maybury Rd | 27.9 | 27.9 | Yes | 25.7 | 21.9 | Yes | 31.9 | 27.1 | Yes |
| B12 | 1 | A901 / A199 Commercial St to A902 / A90 Roundabout | 31.8 | 34.6 | Yes | 31.7 | 29.0 | Yes | 38.4 | 40.0 | Yes |
|  | 2 | A902 / A90 Roundabout to A901 / A199 Commercial St | 32.0 | 40.2 | No | 31.0 | 26.6 | Yes | 32.7 | 26.9 | No |
| B13 | 1 | A720 / A701 Burdiehouse Road to A1 West Slips / Newcraighall Roundabout | 29.5 | 39.9 | No | 26.9 | 29.4 | Yes | 35.7 | 35.9 | Yes |


| Route | Dir | Description | Obs | Mod | AM <br> Within <br> DMRB <br> Criteria | Obs | Mod | IP <br> Within <br> DMRB <br> Criteria | Obs | Mod | PM <br> Within DMRB Criteria |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  |  |  |  |  |  |  |  |
| B14 | 2 | A1 West Slips / Newcraighall Roundabout to A720 / A701 Burdiehouse Road | 42.4 | 32.9 |  | 30.4 | 28.9 | Yes | 34.1 | 37.5 | Yes |
|  | 1 | A720 / A772 Gilmerton Rd to A720 Sheriffhall Roundabout | 21.2 | 23.2 | Yes | 18.4 | 18.0 | Yes | 23.5 | 20.3 | Yes |
|  | 2 | A720 Sheriffhall Roundabout to A720 / A772 Gilmerton Rd | 20.5 | 20.9 | Yes | 16.8 | 18.1 | Yes | 20.4 | 20.1 | Yes |

6.3.3 Table 6.3 and Figure 6.3 (at the end of this chapter) detail the 'Glasgow Area Urban Journey Routes'. Each individual route is illustrated in Appendix G. Table 6.3 also shows the mean observed and modelled journey times for each route in each time period.

Table 6.3 Glasgow Area Urban Journey Routes

| Route | Dir | Description | Obs | Mod | AM <br> Within <br> DMRB <br> Criteria | Obs | Mod | IP <br> Within DMRB Criteria | Obs | Mod | PM <br> Within DMRB Criteria |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  |  |  |  |  |  |  |  |
| C1 | 1 | Port Glasgow - Hillington | 16.9 | 15.0 | Yes | 16.6 | 14.9 | Yes | 17.9 | 15.0 | No |
|  | 2 | Hillington - Port Glasgow | 16.1 | 15.4 | Yes | 14.9 | 15.2 | Yes | 18.5 | 15.4 | No |
| C2 | 1 | Carmyle - Motherwell | 7.5 | 6.4 | Yes | 7.6 | 6.4 | No | 7.7 | 6.4 | No |
|  | 2 | Motherwell - Carmyle | 8.2 | 6.3 | No | 8.2 | 6.3 | No | 8.4 | 6.3 | No |
| C3 | 1 | Irvine - Barrhead | 30.3 | 29.7 | Yes | 28.0 | 28.9 | Yes | 30.1 | 29.2 | Yes |
|  | 2 | Barrhead - Irvine | 28.2 | 29.5 | Yes | 26.1 | 29.2 | Yes | 28.3 | 30.6 | Yes |
| C4 | 1 | East Kilbride Circular (AntiClockwise) | 19.3 | 16.1 | No | 14.8 | 16.1 | Yes | 20.0 | 17.2 | Yes |
|  | 2 | East Kilbride Circular (Clockwise) | 17.1 | 16.3 | Yes | 14.6 | 15.6 | Yes | 16.9 | 15.7 | Yes |
| C5 | 1 | A77 Loganswell Farm Central | 55.2 | 35.8 | No | 36.7 | 30.3 | No | 33.3 | 30.9 | Yes |
|  | 2 | Central - A77 Loganswell Farm | 30.5 | 28.5 | Yes | 31.0 | 29.9 | Yes | 50.7 | 37.8 | No |
| C6 | 1 | M77 J2-Junction with A77 | 5.8 | 6.2 | Yes | 5.9 | 6.2 | Yes | 5.8 | 6.3 | Yes |
|  | 2 | Junction with A77-M77 J2 | 11.6 | 6.3 | No | 5.0 | 6.3 | No | 6.4 | 6.3 | Yes |
| C7 | 1 | A726 Nitshill - A73 Newhouse | 48.3 | 50.4 | Yes | 45.3 | 50.2 | Yes | 49.0 | 52.3 | Yes |
|  | 2 | A73 Newhouse - A726 Nitshill | 58.9 | 52.4 | Yes | 50.4 | 48.3 | Yes | 56.9 | 53.3 | Yes |
| C8 | 1 | Govan - Kingston Bridge | 14.8 | 11.3 | No | 13.5 | 11.2 | No | 17.3 | 11.3 | No |
|  | 2 | Kingston Bridge - Govan | 12.0 | 10.8 | Yes | 13.0 | 10.5 | No | 14.3 | 11.3 | No |


| Route | Dir | Description | Obs | Mod | AM |  | IP |  | PM |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  | Within Obs DMRB Criteria | Mod | Within Obs DMRB Criteria | Mod | Within <br> DMRB <br> Criteria |
| C9 | 1 | A814 Kilpatrick - Hope Street | 25.2 | 22.7 | Yes 22.5 | 20.9 | Yes 23.7 | 22.6 | Yes |
|  | 2 | Hope Street - A814 Kilpatrick | 23.7 | 22.6 | Yes 22.4 | 22.5 | Yes 24.1 | 28.5 | No |
| C10 | 1 | Dumbarton Road - Great Western Road | 3.8 | 3.2 | Yes 3.5 | 3.7 | Yes 3.8 | 3.2 | Yes |
|  | 2 | Great Western Road - <br> Dumbarton Road | 3.5 | 3.0 | Yes 3.3 | 3.5 | Yes 3.4 | 3.0 | Yes |
| C11 | 1 | Johnstone - Bellahouston | 27.9 | 25.5 | Yes 26.1 | 24.4 | Yes 28.4 | 24.6 | Yes |
|  | 2 | Bellahouston - Johnstone | 32.4 | 27.1 | No 27.2 | 26.2 | Yes 31.3 | 28.6 | Yes |
| C12 | 1 | A80 Cumbernauld - M8 | 19.8 | 15.1 | No 14.5 | 13.8 | Yes 14.2 | 13.9 | Yes |
|  | 2 | M8 - A80 Cumbernauld | 18.5 | 15.3 | No 13.9 | 14.8 | Yes 16.6 | 15.4 | Yes |
| C14 | 1 | A77-East Kilbride | 12.3 | 3.2 | No 11.6 | 3.2 | No 13.7 | 3.2 | No |
|  | 2 | East Kilbride - A77 | 13.0 | 10.6 | No 12.3 | 10.4 | No 14.3 | 10.6 | No |
| C15 | 1 | A8 - A728 (Cathcart Road) | 37.4 | 41.1 | Yes 36.6 | 33.3 | Yes 37.1 | 43.1 | No |
|  | 2 | A728 (Cathcart Road) - A8 | 38.2 | 36.9 | Yes 35.6 | 33.0 | Yes 44.6 | 37.2 | No |
| C16 | 1 | Kingsway - Anniesland Cross | 4.7 | 3.6 | No 4.3 | 3.6 | Yes 4.4 | 3.8 | Yes |
|  | 2 | Anniesland Cross - Kingsway | 4.0 | 3.8 | Yes 3.7 | 3.7 | Yes 3.9 | 3.8 | Yes |
| C17 | 1 | A803 Springburn Circular (Anti-Clockwise) | 46.3 | 51.3 | Yes 43.8 | 47.1 | Yes 45.4 | 50.5 | Yes |
|  | 2 | A803 Springburn Circular (Clockwise) | 54.7 | 53.6 | Yes 48.9 | 47.0 | Yes 50.1 | 50.9 | Yes |
| C18 | 1 | Partick - Hillfoot | 14.3 | 15.1 | Yes 11.8 | 15.0 | No 14.3 | 18.1 | No |
|  | 2 | Hillfoot - Partick | 16.9 | 15.7 | Yes 12.4 | 12.9 | Yes 14.5 | 14.2 | Yes |
| C19 | 1 | M77 J2-A8 Bargeddie | 26.3 | 14.7 | No 16.9 | 14.6 | Yes 33.0 | 16.6 | No |
|  | 2 | A8 Bargeddie - M77 J2 | 32.4 | 19.5 | No 16.0 | 16.4 | Yes 43.6 | 25.1 | No |
| C20 | 1 | Glasgow - Bearsden | 13.3 | 13.1 | Yes 14.8 | 12.5 | No 18.6 | 20.1 | Yes |
|  | 2 | Bearsden - Glasgow | 19.0 | 17.7 | Yes 14.5 | 13.2 | Yes 15.4 | 13.5 | Yes |
| C21 | 1 | A82 / A898 Junction Circular (Clockwise) | 50.4 | 46.5 | Yes 47.3 | 46.2 | Yes 51.6 | 47.6 | Yes |
|  | 2 | A82 / A898 Junction Circular (Anti-Clockwise) | 52.2 | 47.1 | Yes 49.7 | 46.0 | Yes 50.8 | 46.8 | Yes |
| C22 | 1 | Great Western Road (M8 to A8014) | 22.6 | 25.7 | Yes 27.7 | 21.0 | No 28.6 | 32.5 | Yes |
|  | 2 | Great Western Road (A8014 to M8) | 31.0 | 27.1 | Yes 25.4 | 19.4 | No 27.6 | 26.4 | Yes |
| C23 | 1 | A725 Blantyre - Coatbridge | 14.7 | 15.2 | Yes 10.8 | 12.0 | Yes 25.7 | 12.7 | No |
|  | 2 | Coatbridge - A725 Blantyre | 15.7 | 15.0 | Yes 16.3 | 13.1 | No 16.3 | 18.4 | Yes |
| C24 | 1 | Bearsden - Kilsyth | 31.2 | 27.8 | Yes 29.7 | 28.2 | Yes 31.6 | 27.6 | Yes |
|  | 2 | Kilsyth - Bearsden | 30.7 | 28.7 | Yes 30.0 | 29.0 | Yes 45.9 | 29.7 | No |


| Route | Dir | Description | Obs | Mod | AM <br> Within <br> DMRB <br> Criteria | Obs | Mod | IP |  | Mod | PM <br> Within <br> DMRB <br> Criteria |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  |  |  |  | Within DMRB Criteria | Obs |  |  |
| C25 | 1 | A807-A814 Partick | 19.4 | 24.1 | No | 20.5 | 16.8 | No | 27.0 | 27.4 | Yes |
|  | 2 | A814 Partick - A807 | 20.8 | 21.4 | Yes | 18.8 | 21.3 | Yes | 27.3 | 33.2 | No |
| C27 | 1 | A71/ A78 Irvine - A73 | 75.2 | 63.4 | No | 63.3 | 63.4 | Yes | 74.4 | 65.5 | Yes |
|  |  | Newhouse |  |  |  |  |  |  |  |  |  |
|  | 2 | A73 Newhouse - A71/ A78 Irvine | 66.1 | 64.6 | Yes | 64.5 | 62.6 | Yes | 67.0 | 62.6 | Yes |
| C28 | 1 | Govan - Cambuslang | 24.9 | 28.1 | Yes | 23.2 | 24.9 | Yes | 29.4 | 28.8 | Yes |
|  | 2 | Cambuslang - Govan | 29.6 | 30.5 | Yes | 20.8 | 23.0 | Yes | 27.8 | 27.2 | Yes |
| C29 | 1 | George Square / Castle St (Anti-Clockwise) | 8.8 | 15.2 | No | 10.4 | 13.0 | No | 12.0 | 12.5 | Yes |
| C31 | 1 | Kilsyth - Auchenkilns | 7.6 | 7.7 | Yes | 7.6 | 7.3 | Yes | 8.6 | 7.4 | Yes |
|  |  | Roundabout |  |  |  |  |  |  |  |  |  |
|  | 2 | Auchenkilns Roundabout Kilsyth | 8.0 | 7.7 | Yes | 8.4 | 7.4 | Yes | 8.3 | 8.8 | Yes |
| C32 | 1 | Bogton - Bishopbriggs | 5.9 | 7.1 | No | 6.2 | 7.0 | Yes | 7.2 | 6.7 | Yes |
|  | 2 | Bishopbriggs - Bogton | 7.9 | 6.7 | Yes | 6.3 | 6.9 | Yes | 8.0 | 6.5 | No |
| C33 | 1 | Mollinsburn - Coatbridge | 4.3 | 3.6 | Yes | 4.1 | 3.6 | Yes | 4.2 | 3.6 | Yes |
|  | 2 | Coatbridge - Mollinsburn | 6.5 | 4.9 | No | 4.6 | 4.8 | Yes | 4.9 | 4.9 | Yes |
| D1 | 1 | Bellgrove St to Main St | 14.1 | 12.8 | Yes | 13.9 | 13.5 | Yes | 15.4 | 13.9 | Yes |
|  | 2 | Main St to Bellgrove St | 14.8 | 14.7 | Yes | 15.2 | 12.9 | No | 15.6 | 12.7 | No |
| D2 | 1 | M80 M9 J9 Stirling to J1 Provan | 32.5 | 24.7 | No | 23.2 | 24.0 | Yes | 23.2 | 25.1 | Yes |
|  | 2 | J1 Provan to M80 M9 J9 Stirling | 27.0 | 25.1 | Yes | 23.5 | 23.6 | Yes | 23.6 | 23.8 | Yes |
| D3 | 1 | A803 A80 Haggs to Townhead | 41.1 | 36.9 | Yes | 35.2 | 37.9 | Yes | 38.1 | 37.3 | Yes |
|  | 2 | Townhead to A803 A80 Haggs | 38.0 | 38.9 | Yes | 36.4 | 37.4 | Yes | 41.9 | 36.9 | Yes |
| D4 | 1 | A89 Airdrie to Baillieston Lights | 14.6 | 14.2 | Yes | 13.9 | 13.7 | Yes | 15.6 | 13.5 | Yes |
|  | 2 | Baillieston Lights to A89 Airdrie | 15.8 | 12.2 | No | 14.5 | 12.0 | No | 15.7 | 12.2 | No |
| D5 | 1 | A775 Newhouse to Glasgow Zoo | 15.7 | 15.8 | Yes | 14.7 | 15.4 | Yes | 17.5 | 16.0 | Yes |
|  | 2 | Glasgow Zoo to A775 Newhouse | 18.3 | 14.9 | No | 15.2 | 14.3 | Yes | 16.8 | 14.9 | Yes |
| D6 | 1 | A725 Raith to A89 Coatbridge | 13.1 | 9.1 | No | 9.9 | 8.9 | Yes | 14.2 | 11.4 | No |
|  | 2 | A89 Coatbridge to A725 Raith | 13.4 | 10.4 | No | 7.9 | 9.0 | Yes | 13.1 | 8.6 | No |
| D7 | 1 | A8 Edinburgh Road to Alexander Park St | 14.6 | 13.9 | Yes | 12.6 | 14.3 | Yes | 15.6 | 14.4 | Yes |
|  | 2 | A8 Alexander Park St to Edinburgh Road | 13.8 | 13.1 |  | 12.0 | 12.4 | Yes | 13.2 | 12.0 | Yes |


| Route | Dir | Description | Obs | Mod | AM <br> Within <br> DMRB <br> Criteria | Obs | Mod | IP <br> Within DMRB Criteria | Obs | Mod | PM <br> Within DMRB Criteria |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| D8 | 1 | A89 Baillieston Lights to Millerston Street | 14.0 | 11.9 | Yes | 12.7 | 12.1 | Yes | 15.2 | 12.3 | No |
|  | 2 | A89 Millerston Street to Baillieston Lights | 15.2 | 14.5 | Yes | 13.2 | 13.6 | Yes | 14.1 | 13.3 | Yes |
| D9 | 1 | A74 Glasgow Zoo to A74 Fielden Street | 11.4 | 9.0 | No | 10.6 | 10.0 | Yes | 12.5 | 11.9 | Yes |
|  | 2 | A74 Fielden Street to Glasgow Zoo | 12.0 | 14.5 | No | 10.5 | 9.3 | Yes | 11.2 | 8.6 | No |
| D10 | 1 | A724 East Kilbride <br> Expressway to A724 <br> Springfield Road | 19.9 | 18.8 | Yes | 18.3 | 20.7 | Yes | 22.6 | 20.3 | Yes |
|  | 2 | A724 Springfield Road to A724 East Kilbride Expressway | 21.3 | 18.4 | Yes | 18.0 | 18.0 | Yes | 19.7 | 16.6 | No |
| D11 | 1 | A8 M8 J6 Newhouse to M8 J13 Provan | 12.9 | 13.1 | Yes | 12.0 | 12.7 | Yes | 13.1 | 14.2 | Yes |
|  | 2 | M8 J13 Provan to A8 M8 Newhouse | 16.5 | 13.0 | No | 12.5 | 12.3 | Yes | 13.8 | 12.8 | Yes |
| E1 | 1 | M8 Junction 29 to M8 Junction 22 | 8.8 | 7.2 | No | 7.9 | 7.0 | Yes | 9.8 | 7.0 | No |
| E2 | 1 | M8 Junction 15 to M8 Junction 24 | 10.5 | 9.3 | Yes | 7.4 | 6.2 | No | 18.6 | 13.5 | No |

6.3.4 Table 6.4 and Figure 6.4 (at the end of this chapter) detail the 'Aberdeen Area Urban Journey Routes'. Each individual route is illustrated in Appendix G. Table 6.3 also shows the mean observed and modelled journey times for each route in each time period.

Table 6.4 Aberdeen Area Urban Journey Routes

| Route | Dir | Description | Obs | Mod | AM <br> Within <br> DMRB <br> Criteria | Obs | Mod | IP <br> Within DMRB <br> Criteria | Obs | Mod | PM <br> Within DMRB <br> Criteria |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  |  |  |  |  |  |  |  |
| A1 | 1 | A90 Slip road at Portlethen to Great Northern Road/B979 | 36.2 | 39.7 | Yes | 29.4 | 28.0 | Yes | 38.7 | 34.7 | Yes |
|  | 2 | Great Northern Road/B979 to A90 slip road at Portlethen | 37.0 | 36.6 | Yes | 27.8 | 25.2 | Yes | 39.2 | 30.6 | No |
| A2 | 1 | A90 Blackdog Junction to A956.A90 | 33.6 | 32.4 | Yes | 22.9 | 20.4 | Yes | 23.0 | 23.8 | Yes |


|  |  |  | AM |  |  |  | IP |  |  |  | PM <br> Within <br> DMRB <br> Criteria |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Route | Dir | Description | Obs | Mod | Within DMRB Criteria | Obs | Mod | Within <br> DMRB Criteria | Obs | Mod |  |
|  | 2 | A956/A90 to A90 Blackdog Junction | 23.5 | 27.2 | No | 23.6 | 20.1 | No | 25.5 | 25.5 | Yes |

6.3.5 Table 6.5 and Figure 6.5 (at the end of this chapter) detail the 'Inter Urban Journey Routes'. Each individual route is illustrated in Appendix G. Table 6.4 also shows the mean observed and modelled journey times for each route in each time period.

Table 6.5 Inter Urban Journey Routes

| Route | Dir | Description | Obs | Mod | AM <br> Within <br> DMRB <br> Criteria | Obs | Mod | IP <br> Within <br> DMRB <br> Criteria | Obs | Mod | PM <br> Within <br> DMRB <br> Criteria |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| B3 | 1 | A912 / A989 to A9 / A811 <br> Roundabout | 43.6 | 48.7 | Yes | 43.4 | 46.3 | Yes | 46.9 | 42.8 | Yes |
|  | 2 | A9 / A811 Roundabout to A85 / A93 | 42.7 | 46.0 | Yes | 43.9 | 47.8 | Yes | 44.5 | 45.7 | Yes |
| B4 | 1 | M9 J10 / A84 to M9 J10 | 50.9 | 49.9 | Yes | 45.9 | 52.8 | Yes | 53.5 | 50.4 | Yes |
|  | 2 | M9 J10 to M9 J10 / A84 | 50.7 | 48.4 | Yes | 44.3 | 51.1 | No | 54.2 | 48.1 | Yes |
| B5 | 1 | M80 J5 / M876 to M8 | 29.4 | 30.1 | Yes | 29.2 | 29.4 | Yes | 29.2 | 29.6 | Yes |
|  | 2 | Hermiston Gate Roundabout <br> M8 Hermiston Gate <br> Roundabout to M80 J5 / M876 | 29.9 | 28.9 | Yes | 29.8 | 28.7 | Yes | 30.4 | 29.4 | Yes |
| B6 | 1 | A985 / A876 to M90 / A9 / A93 Roundabout | 48.0 | 49.3 | Yes | 46.7 | 49.2 | Yes | 49.2 | 49.2 | Yes |
|  | 2 | M90 / A9 / A93 Roundabout to A985 / A876 | 48.8 | 49.2 | Yes | 48.7 | 49.1 | Yes | 55.8 | 49.0 | Yes |
| B7 | 1 | M9 J1a NB Off Slip to A929 / A972 / A90 Dumbbell | 74.4 | 75.3 | Yes | 76.1 | 73.1 | Yes | 75.5 | 83.8 | Yes |
|  |  | Roundabout (West <br> Roundabout) |  |  |  |  |  |  |  |  |  |
|  | 2 | A929 / A972 / A90 Dumbbell <br> Roundabout (West <br> Roundabout) to M9 J1a NB Off <br> Slip | 76.9 | 74.9 | Yes | 79.3 | 70.0 | Yes | 79.9 | 73.5 | Yes |
| B9 | 1 | Newbridge Interchange (A8 / M9 / M8) to M8 J6 / A73 Roundabout | 22.5 | 22.4 | Yes | 22.3 | 22.1 | Yes | 22.7 | 22.3 | Yes |
|  | 2 | M8 J6 / A73 Roundabout to Newbridge Interchange (A8 / M9 / M8) | 24.8 | 22.6 | Yes | 22.7 | 22.4 | Yes | 25.3 | 22.7 | Yes |


| Route | Dir | Description | Obs | Mod | AM <br> Within <br> DMRB <br> Criteria | Obs | IP |  |  | Mod | PM <br> Within <br> DMRB <br> Criteria |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  |  |  | Mod | Within <br> DMRB <br> Criteria | Obs |  |  |
| B10 | 1 | A713 Whitletts Road / B749 | 27.0 | 25.6 | Yes | 25.5 | 25.6 | Yes | 28.4 | 25.5 | Yes |
|  |  | Craigie Road to A77 / B764 |  |  |  |  |  |  |  |  |  |
|  | 2 | A77 / B764 to A713 Whitletts | 24.5 | 26.5 | Yes | 24.3 | 26.3 | Yes | 25.4 | 28.3 | Yes |
|  |  | Road / B749 Craigie Road |  |  |  |  |  |  |  |  |  |

6.3.6 It should be taken into consideration that the journey time routes used in the validation process, except for route 'E', are from TMfS:02 and have not been factored to a 2005 base level. In addition, some of the journey time routes have been physically altered as a result of newly constructed Highway schemes, an example being Route B10 between the A713 Whitletts Road / B749 Craigie Road and A77 / B764. The M77 Extension between Fenwick and Malletsheugh has been completed and included in the network. This will therefore have an effect on the network flows and the journey time.
6.3.7 The journey time routes all have sufficient surveyed data to provide a range of acceptable journey times assuming that a 95\% confidence interval could be expected for each route and that the journey times would vary in the form of a normal distribution. It should be noted that the journey times are unlikely to form a normal distribution but this assumption provides a valuable means for comparing the modelled and observed data.
6.3.8 The confidence intervals used were calculated using the following formula:

95\% Confidence Interval for Population = Sample Mean +- ( $\mathrm{t}(0.025, \mathrm{n}-1)$ * s$)$
95\% Confidence Interval for Mean = Sample Mean +- ( $\left.\mathrm{t}(0.025, \mathrm{n}-1)^{*} \mathrm{~s}\right) /(\sqrt{ } \mathrm{n})$
where:
n - sample size;
t - two tailed t -test with $5 \%$ level of significance and $\mathrm{n}-1$ degrees of freedom; and
$s$ - standard deviation of sample.
6.3.9 It should be noted that lower confidence limits have been capped at zero, ie there are no negative journey times. Appendix $H$ contains detailed journey time analysis for each route detailed in Tables 6.2 to 6.5 , which shows the modelled times versus the observed data along with the confidence intervals.
6.3.10 Comparison between modelled and observed journey times has been carried out in line with DMRB validation acceptability guidelines (Volume 12, Section 2, Part 1, Table 4.2, Criteria 6). The modelled journey times have also been compared with the observed range of surveyed journey times. Table 6.6 below summarises the journey time validation for TMfS:05A.

Table 6.6 TMfS:05A Journey Time Validation

| Area | Total No. of routes | AM <br> No. of routes | \% of routes | IP <br> No. of routes | \% of routes | PM <br> No. of routes | \% of routes |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Number within DMRB criteria - modelled within 15\% observed or 1 minute |  |  |  |  |  |  |  |
| Edinburgh Urban | 14 | 7 | 50\% | 12 | 86\% | 10 | 71\% |
| Glasgow Urban | 83 | 56 | 67\% | 65 | 78\% | 55 | 66\% |
| Aberdeen Urban | 4 | 3 | 75\% | 3 | 75\% | 3 | 75\% |
| Inter Urban | 14 | 14 | 100\% | 13 | 93\% | 14 | 100\% |
| Number within range of observed values |  |  |  |  |  |  |  |
| Edinburgh Urban | 14 | 8 | 57\% | 7 | 50\% | 5 | 36\% |
| Glasgow Urban | 81 | 39 | 48\% | 29 | 36\% | 32 | 40\% |
| Aberdeen Urban | 4 | 3 | 75\% | 3 | 75\% | 3 | 75\% |
| Inter Urban | 14 | 6 | 43\% | 5 | 36\% | 4 | 29\% |
| Number within 95\% confidence interval of population |  |  |  |  |  |  |  |
| Edinburgh Urban | 14 | 12 | 86\% | 10 | 71\% | 11 | 79\% |
| Glasgow Urban | 83 | 67 | 81\% | 57 | 69\% | 60 | 72\% |
| Aberdeen Urban | 4 | 4 | 100\% | 4 | 100\% | 3 | 75\% |
| Inter Urban | 14 | 10 | $71 \%$ | 9 | 64\% | 7 | 50\% |
| Number within 95\% confidence interval of mean |  |  |  |  |  |  |  |
| Edinburgh Urban | 14 | 4 | 29\% | 5 | 36\% | 5 | 36\% |
| Glasgow Urban | 83 | 51 | 61\% | 35 | 42\% | 37 | 45\% |
| Aberdeen Urban | 4 | 3 | 75\% | 1 | 25\% | 3 | 75\% |
| Inter Urban | 14 | 6 | 43\% | 4 | 29\% | 3 | 21\% |

6.3.11 Inspection of Table 6.6 and Appendix $H$ shows that the journey time validation for TMfS:05A is broadly similar to TMfS:05 and demonstrates a reasonable level of validation.
6.3.12 As the Inter Urban Routes are surveyed over longer distances, additional analysis was undertaken where these routes were divided into segments. Table 6.7 details these segments with the diagrams and results shown in Appendix I. Overall, the journey time segments show as good a level of validation as exhibited over the whole route.

Table 6.7 Inter Urban Route segments

| Route | Segment | Description |
| :---: | :---: | :---: |
| B3 | 1 | A912/A989 - A9/M90 Roundabout |
|  | 2 | A9/M90 Roundabout - A9/B8033 |
|  | 3 | A9/B8033 - A9/A811 Roundabout |
| B4 | 1 | M9 J10/A84 - A907/A977 |
|  | 2 | A907/A977 - M876/A905 Roundabout |
|  | 3 | M876/A905 Roundabout - M9 J10 |
| B5 | 1 | M80 J5/M876-M9 J7 NB On slip |
|  | 2 | M9 J7 NB On slip - M9 J3 Slips |
|  | 3 | M9 J3 Slips - M8 Hermiston Gate Roundabout |
| B6 | 1 | A985/A876 Roundabout - M90 J2 NB Off slip |
|  | 2 | M90 J2 NB Off slip - M90 J8 NB Off slip |
|  | 3 | M90 J8 NB Off slip - M90/A9/A93 Roundabout |
| B7 | 1 | M9 J1a NB Off Slip - A92/B9149 West Slips |
|  | 2 | A92/B9149 West Slips - A91/A92 |
|  | 3 | A929 / A972 / A90 Dumbbell Roundabout (West Roundabout) |
| B9 | 1 | A8/M9/M8 Newbridge GSJ - M8 J3 WB Off Slip |
|  | 2 | M8 J3 WB Off Slip - M8 J4 East Slips |
|  | 3 | M8 J4 East Slips - M8 J6/A73 Roundabout |
| B10 | 1 | A713 Whitletts Road/B749 Craigie Road - A77/A78 Roundabout |
|  | 2 | A77/A78 Roundabout - A77/B7038 NB On Slip |
|  | 3 | A77/B7038 NB On Slip - A77/B764 |

### 6.4 Validation Count Sites

6.4.1 Traffic count data not used in calibration has been used for the purposes of the validation and the same data used in TMfS:05 has been used when validating TMfS:05A. In total, 1,372 one-way counts have been used to present the validation of the HAM. The locations of these sites are described in Appendix J along with the source, type and date of the associated count. Figure 6.5 provides an illustration of the independent validation counts within the TMfS study area.
6.4.2 Appendix K presents tables for the AM peak, Inter-Peak and PM peak observed and modelled flows. The GEH statistic has again been used to assess the overall acceptability of the results.
6.4.3 Table 6.8 presents a summary of the validation site analysis:

## Table 6.8 Validation Site Analysis

| \% of sites with GEH value (TMfS:05 values in brackets) |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: |
| Time | $\leq 5$ | $\leq 7$ | $\leq 10$ | $\leq 12$ | $\leq 15$ |
| Period |  |  |  |  |  |
| AM | $48(48)$ | $65(65)$ | $82(84)$ | $89(92)$ | $97(98)$ |
| IP | $60(60)$ | $75(77)$ | $90(91)$ | $95(97)$ | $99(100)$ |
| PM | $49(50)$ | $65(65)$ | $84(84)$ | $92(93)$ | $98(99)$ |
| Target | $60 \%$ | $80 \%$ | $95 \%$ | $100 \%$ |  |

6.4.4 As with the link flow analysis performed on the calibration sites the majority of sites exhibit a GEH statistic less than 12. However, the highest GEH values are 27.9 in the AM peak, 21.0 in the Inter-Peak and 29.5 in the PM peak respectively. Appendix $L$ contains graphical representations of the screenline results. Of all the Key Links with a GEH in excess of 15, there are 39 in the AM peak, 13 in the Inter-Peak and 34 in the PM peak.

### 6.5 Trip Length Distribution Analysis

6.5.1 Trip Length Distribution analysis has also been undertaken for each vehicle class. Appendix M contains the trip length distributions for 'Car In Work', 'Car Non Work', 'LGV' and 'HGV' for the AM peak, Inter-Peak and PM peak respectively.
6.5.2 For each graph there are two trip length distributions shown. The first is the TMfS Prior matrix (Prior). The second is the Final TMfS assignment matrix after matrix estimation (Estimated).
6.5.3 The matrix estimation process has produced a slight increase in short distance trips in comparison to the prior matrices. This can be expected from simple matrix estimation techniques, as MVESTM adds in a small number of short distance trips particularly to match calibration screenline counts, especially those with a higher assigned confidence level. This slight increase is not deemed to reduce the quality of the calibrated matrix.

### 6.6 HGV Screenline Analysis

6.6.1 HGV Screenline Analysis has also been undertaken for the screenlines used in the validation where suitable classified vehicle count data was available.
6.6.2 Table 6.9 presents a summary of the HGV screenline analysis:

## Table 6.9 HGV Screenline Analysis

| \% of sites with GEH value (TMfS:05 values in brackets) |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: |
| Time | $\leq 5$ | $\leq 7$ | $\leq 10$ | $\leq 12$ | $\leq 15$ |
| Period |  |  |  |  |  |
| AM | $63(54)$ | $77(68)$ | $88(82)$ | $92(92)$ | $96(93)$ |
| IP | $66(57)$ | $79(70)$ | $90(85)$ | $93(91)$ | $96(96)$ |
| PM | $66(59)$ | $78(73)$ | $90(88)$ | $95(94)$ | $98(97)$ |
| Target | $60 \%$ | $80 \%$ | $95 \%$ | $100 \%$ |  |

6.6.3 The majority of HGV screenlines exhibit a GEH statistic of less than 12 . The highest GEH value in the AM peak is 26.0, with corresponding figures of 28.1 and 32.1 for the Inter-Peak and PM peak periods respectively. It should be stressed that no specific calibration work is carried out on HGVs, only on total vehicles and so all HGV data is used for validation. For all of the Key Links with a GEH statistic greater than 15, there are 46 in the AM peak, 44 in the Inter-Peak and 19 in the PM peak.
6.6.4 The majority of sites exhibit a GEH statistic of less than 12. As previously mentioned, the number of screenline sites used in the validation process has significantly increased with many of these additional counts being in rural areas or on the periphery of the modelled area, which can be affected by a lack of intra-zonal trips.
6.6.5 Comparison of the TMfS:05 and TMfS:05A GEH statistics shows some changes in the level of calibration and this is principally due to the update of demand matrices, and particularly the matrix estimation process.
6.6.6 Appendix $N$ contains graphical representations of TMfS:05A screenline results, showing counts versus modelled flow in vehicles. This shows that TMfS contains a good match for most HGV counts, however, the model underestimates some links with high HGV flows.

### 6.7 Car In Work, Car Non Work Analysis

6.7.1 Screenline analysis was also conducted for those sites where a count was available for both the 'Car In Work' and 'Car Non Work' journey purposes, these were from RSI sites where trip purpose had been one of the questions. Tables 6.10 and 6.11 show the screenline analysis for 'Car In Work' and 'Car Non Work' respectively.

Table 6.10

| \% of sites with GEH value (TMfS:05 values in brackets) |  |  |  |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | :---: | :---: | :---: | :---: |
| Time | $\leq 5$ | $\leq 7$ | $\leq 10$ | $\leq 12$ | $\leq 15$ |  |  |  |  |
| Period |  |  |  |  |  |  |  |  |  |
| AM | $72(74)$ | $83(86)$ | $93(93)$ | $97(97)$ | $98(100)$ |  |  |  |  |
| IP | $80(78)$ | $90(91)$ | $96(98)$ | $96(98)$ | $98(100)$ |  |  |  |  |
| PM | $74(80)$ | $86(88)$ | $94(95)$ | $97(98)$ | $98(99)$ |  |  |  |  |
| Target | $60 \%$ | $80 \%$ | $95 \%$ | $100 \%$ |  |  |  |  |  |

6.7.2 The majority of sites exhibit a GEH statistic less than 12.

Table 6.11 Car Non Work Screenline Analysis

| \% of sites with GEH value (TMfS:05 values in brackets) |  |  |  |  |  |
| :--- | :---: | :---: | :---: | :---: | ---: |
| Time | $\leq 5$ | $\leq 7$ | $\leq 10$ | $\leq 12$ | $\leq 15$ |
| Period |  |  |  |  |  |
| AM | $63(61)$ | $78(74)$ | $89(86)$ | $93(91)$ | $96(95)$ |
| IP | $67(67)$ | $85(81)$ | $94(95)$ | $96(97)$ | $98(98)$ |
| PM | $63(53)$ | $77(66)$ | $91(83)$ | $95(88)$ | $96(93)$ |
| Target | $60 \%$ | $80 \%$ | $95 \%$ | $100 \%$ |  |

6.7.3 The majority of sites exhibit a GEH statistic less than 12.
6.7.4 Both the 'Car In Work' and 'Car Non Work' screenline analysis compare favourably with TMfS:05 results.
6.7.5 In a similar comparison to the HGV validation, it should be noted that Total vehicles are used in calibration and therefore all data relating to the In Work and Non Work split is used in validation. The resulting analysis is not generally valid to compare to screenline based targets, but more so demonstrative of the validation a combination of matrix splitting (into In Work and Non Work) and the assignment methodology.

### 6.8 Census Travel to Work Data

6.8.1 The post MVESTM TMfS:05A AM peak hour matrix has been validated against 'Census Travel-to-Work' data. Table 6.12 shows the pattern, as a percentage of the total, of productions and attractions in both TMfS:05 and in the 'Census Travel-to-Work' AM peak hour matrices.
6.8.2 TMfS:05 tends to have slightly high proportions in the urban areas and much smaller proportions in the more rural areas. This is because within urban areas, TMfS has a fine zoning system; rural areas however, have a coarse zoning system. In these local
authorities, the only trips in the model are long distance trips and intra zonal trips are not included.
6.8.3 The table shows that the pattern within the base AM peak TMfS:05A matrix demonstrates a good match with the Census Travel-to-Work matrix.

Table 6.12 Production and Attraction patterns as a percentage of the total

| Local Authority | Census productions | TMfS:05A productions | Census attractions | TMfS:05A attractions |
| :---: | :---: | :---: | :---: | :---: |
| Aberdeenshire | 4\% | 2\% | 2\% | 1\% |
| Angus | 2\% | 1\% | 1\% | 1\% |
| Argyll \& Bute | 0\% | 0\% | 0\% | 0\% |
| City of Aberdeen | 5\% | 7\% | 8\% | 8\% |
| City of Dundee | 3\% | 2\% | 4\% | 3\% |
| City of Edinburgh | 11\% | 13\% | 14\% | 15\% |
| City of Glasgow | 10\% | 17\% | 16\% | 22\% |
| Clackmannanshire | 1\% | 1\% | 1\% | 1\% |
| Dumfries \& Galloway | 2\% | 2\% | 2\% | 2\% |
| East Ayrshire | 3\% | 2\% | 2\% | 2\% |
| East Dunbartonshire | 4\% | 4\% | 2\% | 2\% |
| East Lothian | 2\% | 1\% | 1\% | 1\% |
| East Renfrewshire | 3\% | 4\% | 1\% | 2\% |
| England \& Wales | 0\% | 0\% | 0\% | 0\% |
| Falkirk | 4\% | 3\% | 3\% | 2\% |
| Fife | 8\% | 8\% | 7\% | 8\% |
| Highland | 0\% | 0\% | 0\% | 0\% |
| Inverclyde | 2\% | 0\% | 2\% | 1\% |
| Midlothian | 2\% | 1\% | 1\% | 1\% |
| Moray | 0\% | 0\% | 0\% | 0\% |
| North Ayrshire | 3\% | 2\% | 2\% | 2\% |
| North Lanarkshire | 7\% | 6\% | 6\% | 6\% |
| Perthshire \& Kinross | 3\% | 2\% | 3\% | 2\% |
| Renfrewshire | 4\% | 5\% | 5\% | 5\% |
| South Ayrshire | 3\% | 2\% | 3\% | 2\% |
| South Lanarkshire | 7\% | 6\% | 6\% | 5\% |
| Stirling | 2\% | 2\% | 2\% | 2\% |
| The Borders | 1\% | 1\% | 1\% | 1\% |
| West Dunbartonshire | 2\% | 1\% | 1\% | 1\% |
| West Lothian | 4\% | 2\% | 4\% | 2\% |

6.8.4 Appendix $P$ contains similar analysis to Table 6.12, although the data in the Appendix is presented in terms of the pattern of trips produced by each Local Authority. For each Local Authority, the AM peak trip pattern to each of the other Local Authorities demonstrates a good match to the Census Travel-to-Work data. This data is also shown with the exclusion of intra Local Authority Trips. This shows an even better match, for all local authorities except those, right on the model periphery.
6.8.5 It should be noted, however, that the TMfS commuter matrix was extracted from the Base Year Non-Work matrix using factors from the Scottish Household Survey. These factors are only at a three sector level (Edinburgh, Glasgow and elsewhere) and hence the analysis is very coarse. It should, also be noted that the factors tend to be higher in the Glasgow and Strathclyde area.


Figure 6.2 Edinburgh Area Urban Journey Routes
(see Appendix J for details of each route)


Figure 6.3 Glasgow Area Urban Journey Routes
(See Appendix G for details of each route)


Figure 6.4 Aberdeen Area Urban Journey Routes
(See Appendix G for details of each route)


Figure 6.5 Inter Urban Journey Routes
(See Appendix G for details of each route)


Figure 6.6 Validation Count Site Locations

## 7 Conclusions and Recommendations

### 7.1 Conclusions

7.1.1 This report has presented the calibration and validation of the TMfS:05A Rebase Highway Assignment Model.
7.1.2 The network was developed from the equivalent TMfS:05 network with the refinement of the road network and zone system in the Highlands and Argyll and Bute areas and the addition of ferry links and connecting roads infrastructure to represent the Scottish Islands.
7.1.3 New demand data from recent roadside interviews has been incorporated in the highway model in areas where the model was previously perceived to be weak, namely the Ayrshire and Dundee areas.
7.1.4 An exacting calibration has been undertaken to link/screenline counts. The model is particularly well calibrated in the key areas (trunk roads/motorways), it validates well in the vast majority of the modelled area. Whilst it is less well calibrated in some rural areas, due to the large zones on the periphery of the modelled area and absence of quality observed data, the model still meets good standards of calibration.
7.1.5 The model validates well in the key areas against journey times and against the very large number of counts not included in calibration.
7.1.6 Our view is that the HAM has been successfully developed and is fit for its intended purpose.
7.1.7 The TMfS:05A Highway Assignment Model can be used for the assessment of major strategic Highway schemes and policy decisions as part of the TMfS modelling suite. It is also fit for use as a source of travel demand and network structure for more localised models.

### 7.2 Recommendations

7.2.1 For future development, it is recommended that the highway matrices are enhanced using new RSI data. In particular, the collection of RSI data within Edinburgh is particularly dated and the model would benefit from inclusion of updated information.
7.2.2 Each potential application of the model should be assessed in detail prior to ensure that the quality of the model is appropriate for the desired output as the quality of data input and consequently output differs across the entire modelled area.

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